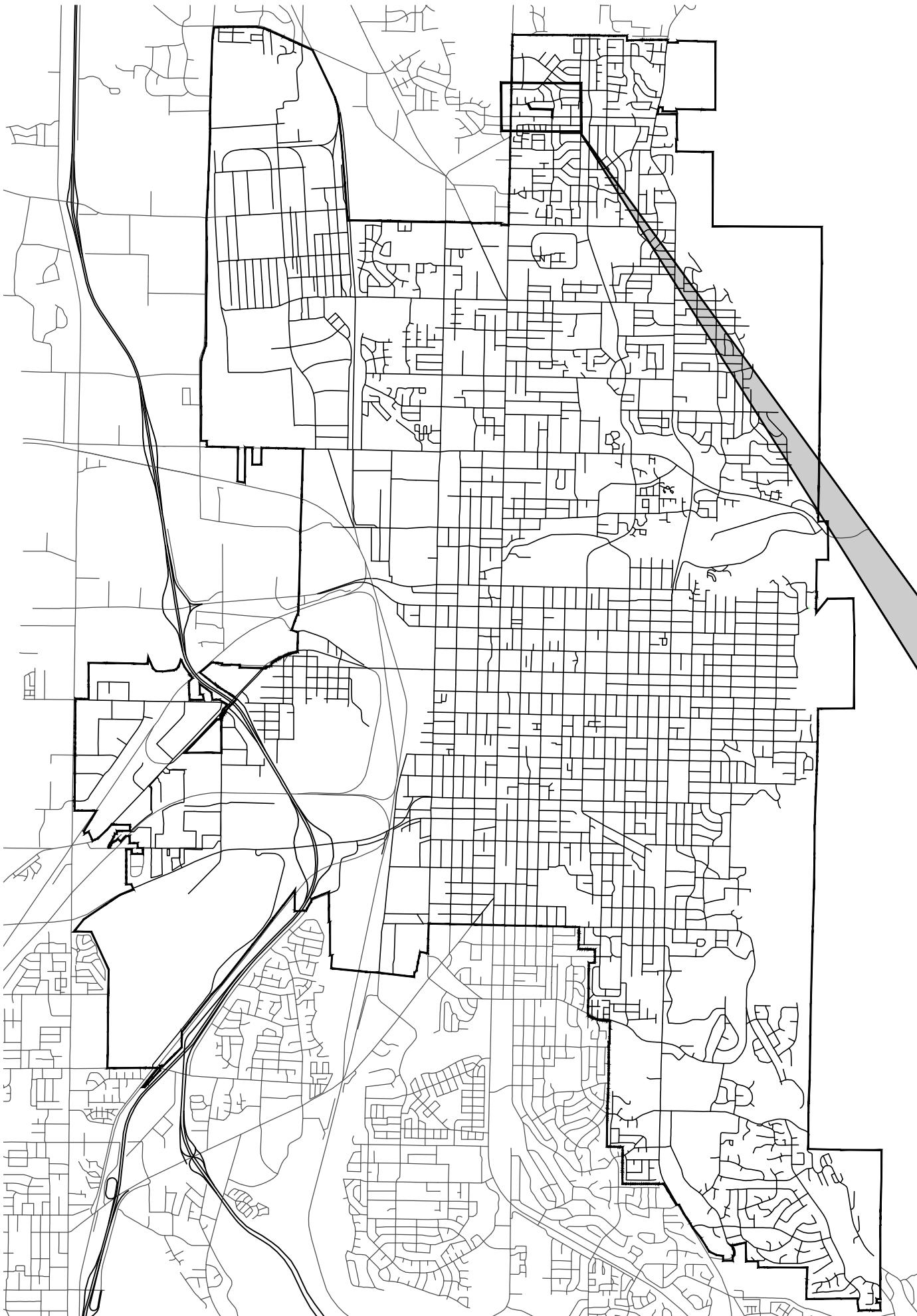


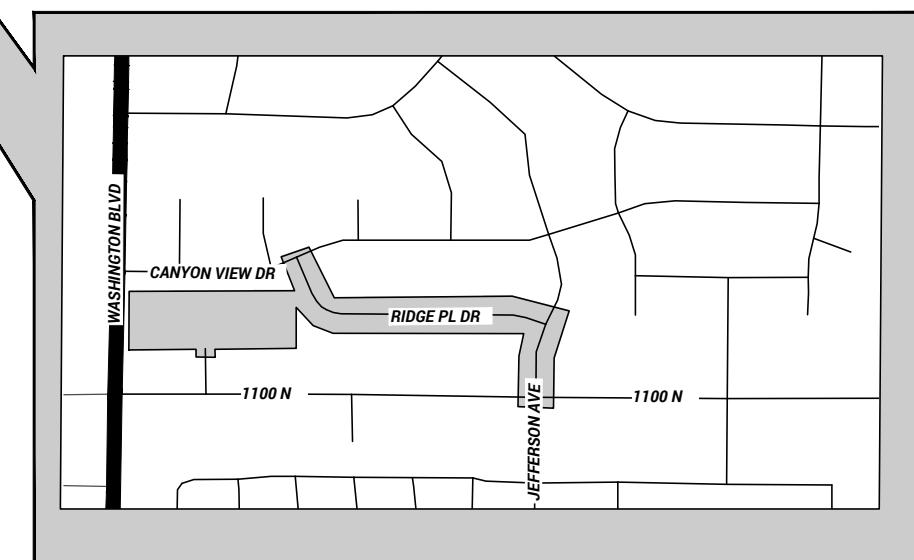
Ogden City Engineering

DETENTION POND TO 1100 N STORM PROJECT

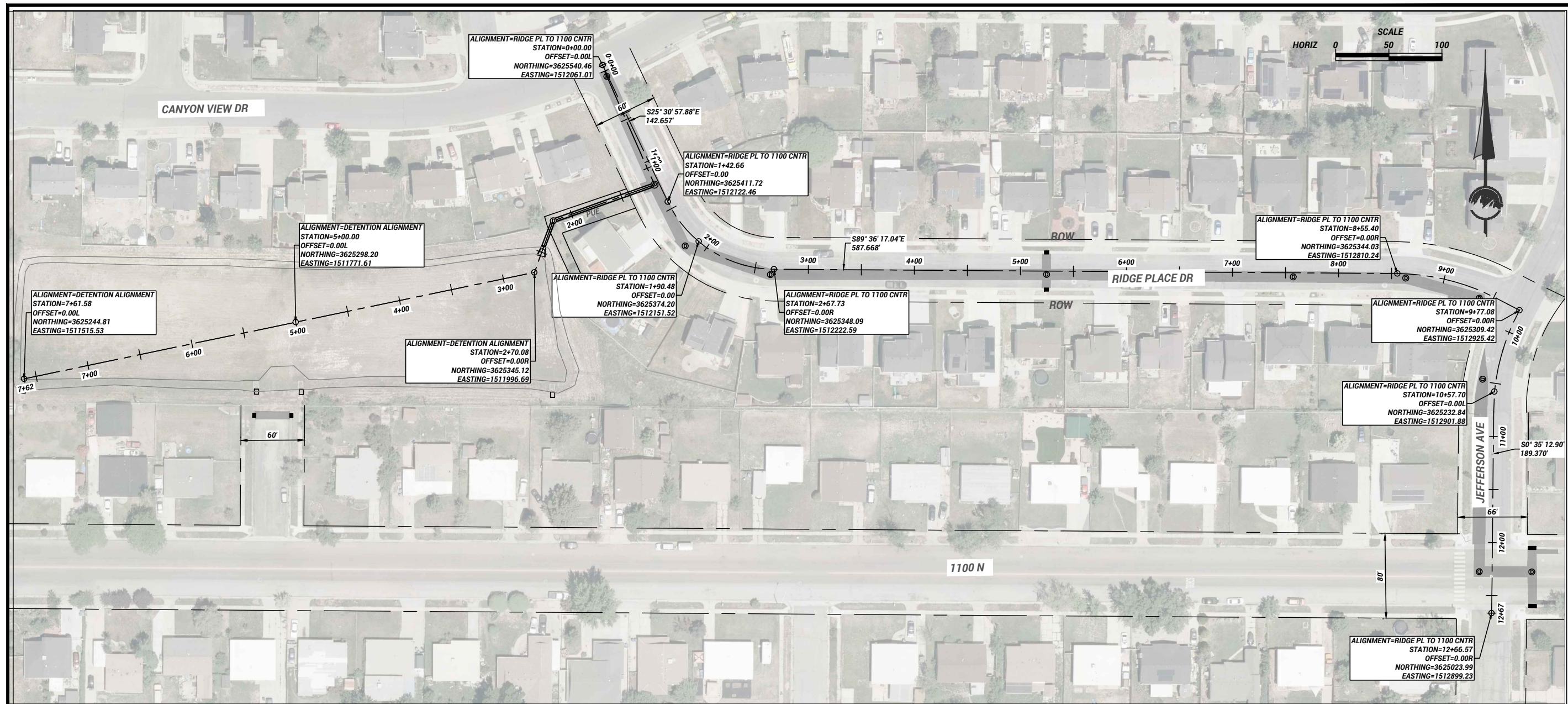


SHEET INDEX

PG	TITLE	DESCRIPTION
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4-7	SD2-SD5	STORM DESIGN
8-9	DT1-D2	DETAILS
10-11	S001 -S002	GENERAL WEIR STRUCTURE NOTES
12	S101	WEIR STRUCTURE PLAN AND SECTION
13	S901	WEIR STRUCTURE DETAILS



G1		0	REVISION
SHEET		1	
REV. DATE DESCRIPTION			
0 MM/DD/YY DESCRIPTION			
PROFESSIONAL ENGINEER SIGNATURE			
No. 9800042-2025 BRADLEY E. MILLER 1/19/26			
STATE OF UTAH * * * * *			
DESIGNED DRAWN CHECKED	P.J.S. P.J.S.	DATE 1/29/2026	
DRAWING SCALE			
H: NONE V: NONE	(22x34) (22x34)	This has measures existing in inches on the original drawing	
PLOT DATE: 1/29/2026 2:58 PM			
DRAWING NAME: G1 Title 1100 N.dwg			
2549 Washington Blvd, Suite 760 Ogden, UT 84401 Phone: 801-629-8980 engineering.ogdencity.com			



GENERAL NOTES

1. ALL WORK SHALL CONFORM TO THE CURRENT EDITION OF THE "MANUAL OF STANDARD SPECIFICATIONS" AND "MANUAL OF STANDARD PLANS" PREPARED BY THE UTAH CHAPTER OF THE AMERICAN PUBLIC WORKS ASSOCIATION AND OGDEN CITY AMENDMENTS TO SUCH STANDARD SPECIFICATIONS AND STANDARD PLANS UNLESS SPECIFIED OTHERWISE.
2. THE CONTRACTOR SHALL FULLY COMPLY WITH THE CONDITIONS OF THE OGDEN CITY ROW PERMIT AND OGDEN CITY ORDINANCES, POLICIES, REGULATIONS, SPECIFICATIONS AND STANDARDS RELATIVE TO WORK IN THE PUBLIC WAY AS APPLICABLE.
3. ALL EXISTING VALVES BEING ABANDONED SHALL HAVE SURFACE IMPROVEMENTS (RISER, COVER, AND CONCRETE COLLAR) REMOVED AND SURFACE REPAIRED.
4. ALL PAVING AND RESURFACING SHALL BE COMPLETE WITHIN 3 CALENDAR DAYS FROM THE TIME OF EXCAVATION UNLESS OTHERWISE APPROVED BY THE CITY ENGINEER.
5. NO BACKFILL SHALL OCCUR UNTIL INSPECTED AND APPROVED.
6. CONTRACTOR SHALL COMPLY WITH ALL FEDERAL, STATE AND LOCAL SAFETY REGULATIONS DURING ALL PHASES OF PROJECT.
7. WORK IN PUBLIC STREETS, ONCE BEGUN, SHALL BE PROSECUTED TO COMPLETION WITH DILIGENCE AND EXPEDITION AND RESTORE THE PUBLIC RIGHT OF WAY TO CITY SPECS WITHOUT DELAY AS TO PROVIDE MINIMUM INCONVENIENCE TO ADJACENT PROPERTY OWNERS AND THE PUBLIC.
8. CONTRACTOR IS RESPONSIBLE TO RESTORE ALL LAND WHETHER PUBLIC OWNERSHIP OR PRIVATE OWNERSHIP, TO ITS ORIGINAL STATE.
9. CONTRACTOR SHALL BE RESPONSIBLE TO PROVIDE TEMPORARY CONSTRUCTION FACILITIES, WATER, POWER, SANITATION, AND TELEPHONE SERVICE AS NEEDED.
10. CONTRACTOR IS RESPONSIBLE TO LOCATE ALL EXISTING WATER, SEWER, ELECTRICAL, GAS AND OTHER UTILITY LINES.
11. CONTRACTOR IS RESPONSIBLE TO SUBMIT AS-BUILT DRAWINGS TO THE CITY PRIOR TO FINAL PAYMENT.
12. ALL MATERIALS AND WORKMANSHIP SHALL BE GUARANTEED FOR A PERIOD OF ONE YEAR FOLLOWING COMPLETION OF WORK. REASONABLE WEAR EXCEPTED.

EXCAVATION NOTES

1. EXCAVATED MATERIAL MAY NOT BE USED AS BACKFILL IN THE PIPE ZONE.
2. EXCAVATION SHALL BE DONE IN ACCORDANCE WITH OSHA REQUIREMENTS (LATEST REVISION).
3. TEMPORARY SHORING OF THE EXCAVATION, IF REQUIRED, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

COORDINATION

1. ALL STORM PIPE MUST BE CLEANED AND CCTV INSPECTED, AND DELIVER CCTV FOOTAGE TO THE CITY PROJECT MANAGER BEFORE ASPHALT IS INSTALLED.
2. IF ANY WATERLINE SHUT DOWNS ARE NEEDED, THEN WE REQUIRE A FIVE WORKING DAYS NOTICE FOR COMMERCIAL AND 48 HOUR NOTICE FOR RESIDENTIAL. CONTRACTOR IS TO COORDINATE WITH OGDEN CITY FOR SHUT DOWN DATES AND TIMES. CONTRACTOR WILL PROVIDE NOTICES AND OGDEN CITY WATER WILL PERFORM VALVE SHUT DOWNS.
3. APPLICATION FOR INSPECTION BY OGDEN CITY ENGINEERING SHALL BE MADE BY THE CONTRACTOR A MINIMUM OF 24 HOURS BEFORE SERVICES ARE REQUIRED.

PIPING NOTES

1. CONTRACTOR IS RESPONSIBLE FOR ALL PIPE FABRICATION AND SIZING (RCP CLASS 3 MINIMUM).
2. MINIMUM COVER REQUIRED SHALL BE 24" IN OGDEN CITY R.O.W.
3. ALL WORK SHALL HAVE 18" MINIMUM CLEARANCE FROM ALL OTHER UTILITIES.
4. ABANDONED STORM PIPES SHALL BE EITHER REMOVED OR CAPPED WITH A CONCRETE PLUG.

UTILITY LOCATION

THE LOCATION OF UNDERGROUND UTILITIES SHOWN ON THESE PLANS IS BASED ON INFORMATION GATHERED FROM UTILITIES AND/OR FROM ABOVE-GROUND STRUCTURES OR EVIDENCE FOUND AT THE TIME OF SURVEY. AS SUCH, THE UNDERGROUND INFORMATION IS A BEST ESTIMATE. OGDEN CITY DOES NOT REPRESENT OR GUARANTEE THAT THE UNDERGROUND INFORMATION PROVIDED IS CORRECT OR UP TO DATE.

IT SHALL BE THE CONTRACTORS FULL RESPONSIBILITY TO CONTACT THE VARIOUS UTILITY COMPANIES TO LOCATE THEIR FACILITIES PRIOR TO PROCEEDING WITH CONSTRUCTION. CALL BLUESTAKES A MINIMUM OF 48 HOURS PRIOR TO BEGINNING ANY DIGGING OR UTILITY WORK.

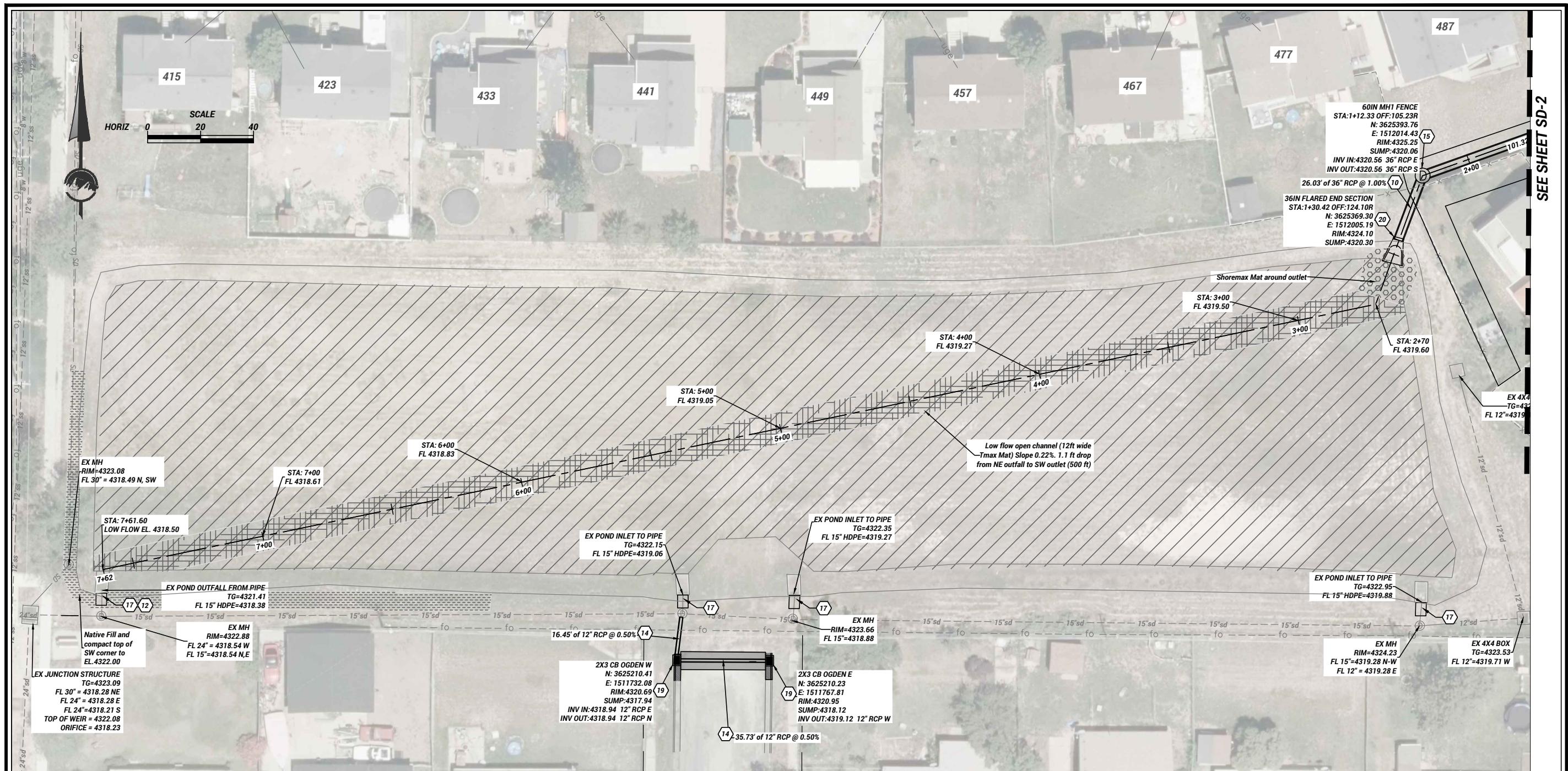
NO ADDITIONAL COMPENSATION SHALL BE PAID TO THE CONTRACTOR FOR DAMAGE AND REPAIR TO THESE FACILITIES CAUSED BY THE CONTRACTORS ACTIONS.

CONTACTS

UTILITY COMPANIES:
BLUE STAKES 800-662-4111
QUESTAR GAS DAN MACDONALD 801-324-3539
ROCKY MOUNTAIN POWER CRAIG BRUESTLE
801-629-4430
OGDEN CITY:
OGDEN CITY ENG. PHIL SUITER 801-629-8971
WATER TED BULLOCK 801-629-8363
STORM KEN MIDDLETON 801-940-6428
SEWER BILL SIMPSON 629-8331
URBAN FORESTER 801-629-8369

NOTES:

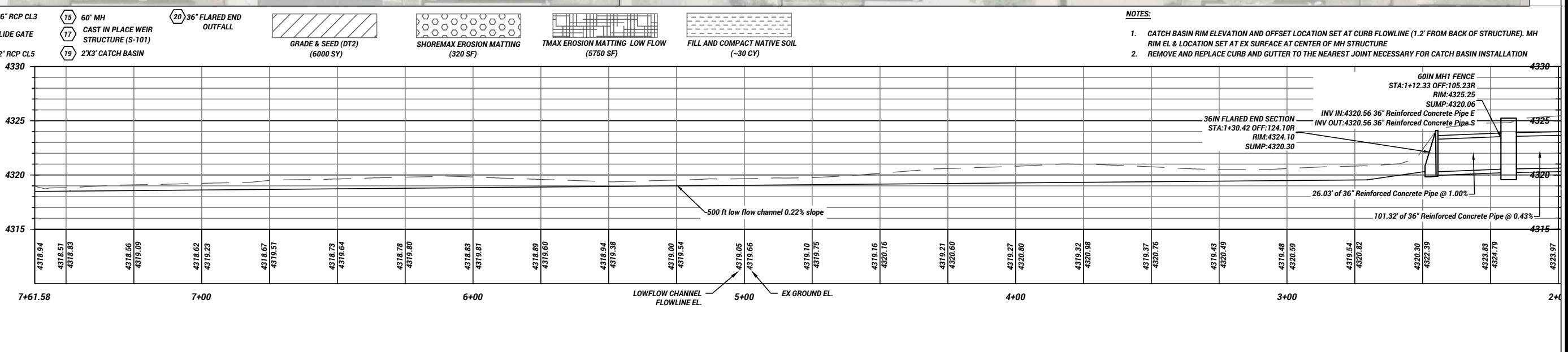
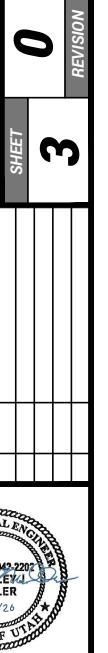
1. THE HORIZONTAL DATUM TO BE USED IS THE NAD 83 STATE PLANE UTAH NORTH ZONE GRID
2. THE VERTICAL DATUM TO BE USED IS NAVD 88 USING GEOD 12A.

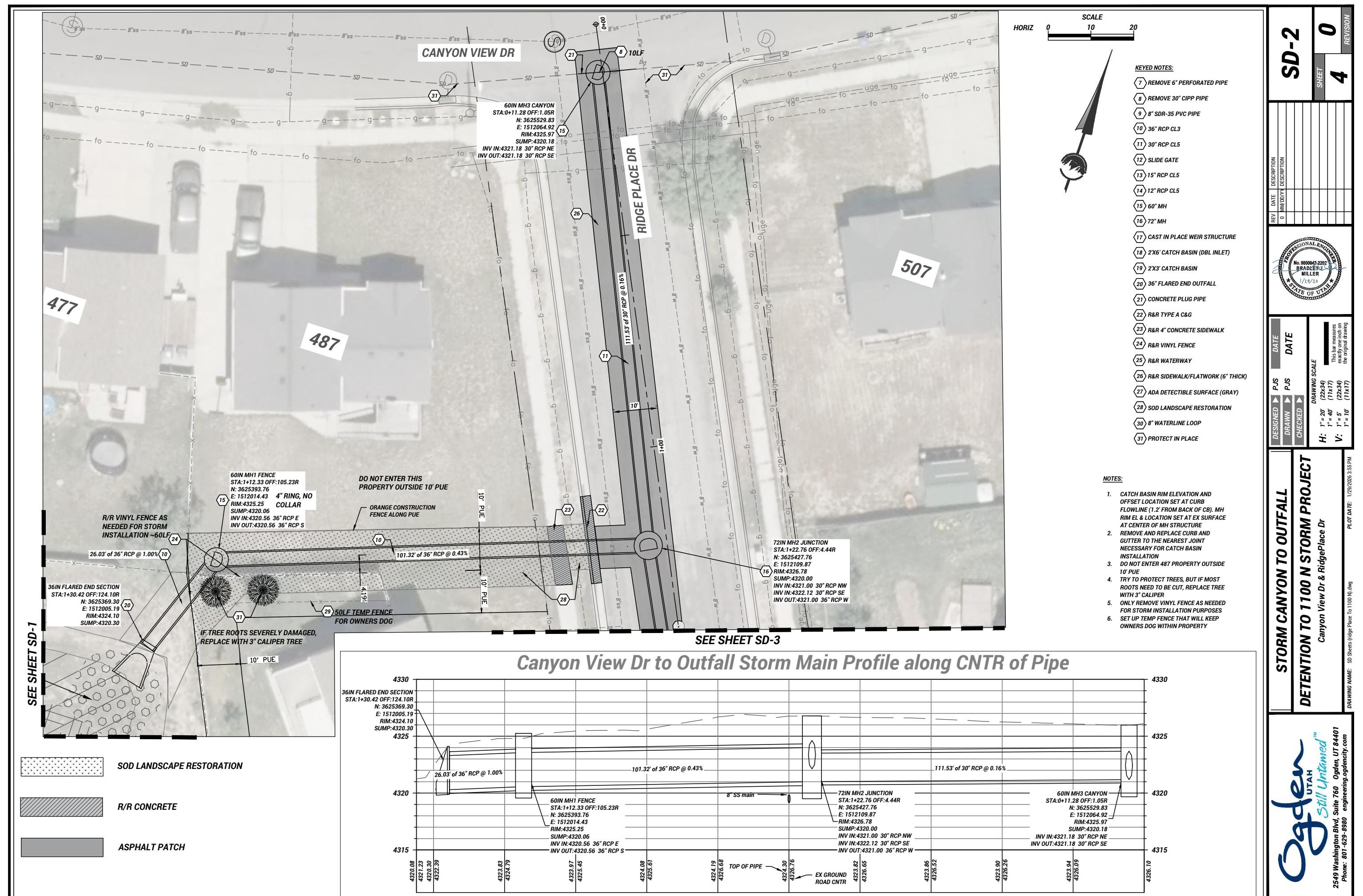


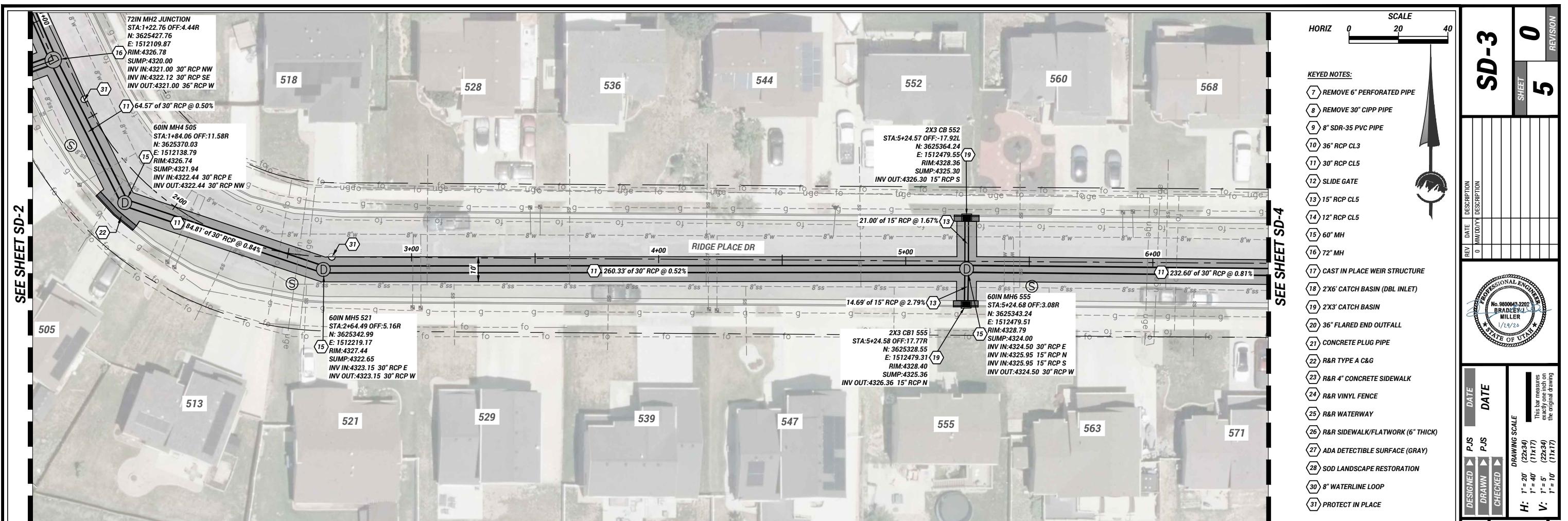
SD-1	0	REVISION
3	0	REVISION
3	0	REVISION
3	0	REVISION

DETENTION POND
420 E 1150 N
PLOT DATE: 1/29/2026 3:46 PM
DRAWING NAME: SD Sheets (ridge Place To 1100 N).dwg

Ogden UTAH Still Untamed™
2549 Washington Blvd, Suite 760 Ogden, UT 84401
Phone: 801-629-8980 engineering.ogdenity.com

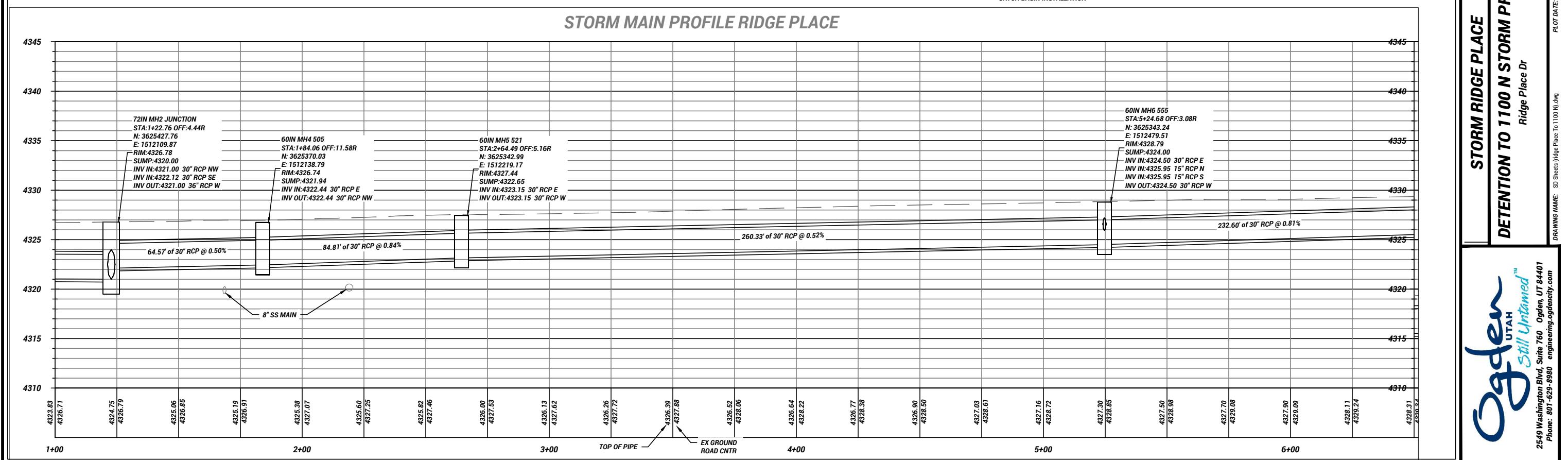


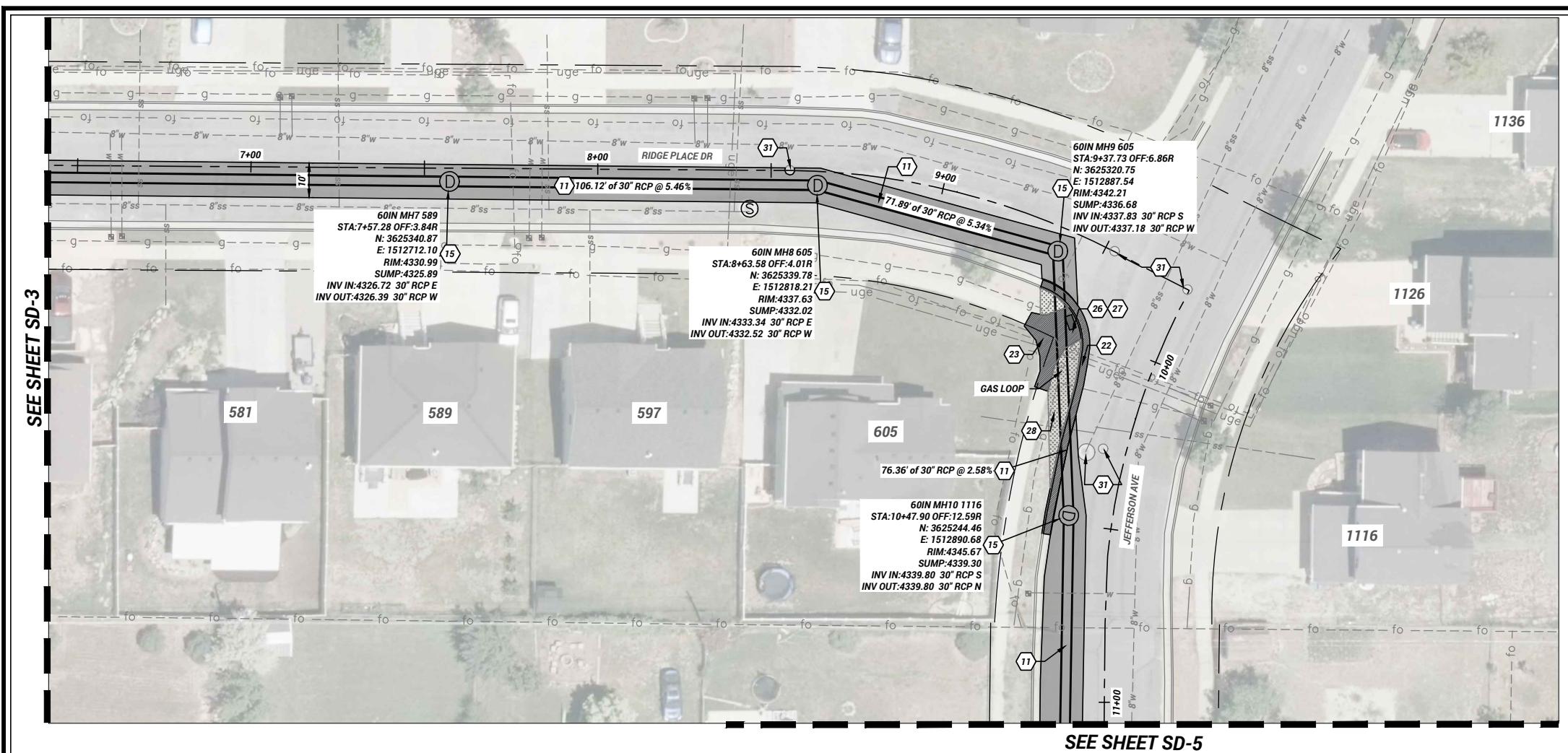




NOTES

1. CATCH BASIN RIM ELEVATION AND OFFSET LOCATION SET AT CURB FLOWLINE (1.2' FROM BACK OF CB). MH RIM EL & LOCATION SET AT EX SURFACE AT CENTER OF MH STRUCTURE
2. REMOVE AND REPLACE CURB AND GUTTER TO THE NEAREST JOINT NECESSARY FOR CATCH BASIN INSTALLATION





KEYED NOTES:

7	REMOVE 6" PERFORATED PIPE
8	REMOVE 30" CIPP PIPE
9	8" SDR-35 PVC PIPE
10	36" RCP CL3
11	30" RCP CL5
12	SLIDE GATE
13	15" RCP CL5
14	12" RCP CL5
15	60" MH
16	72" MH
17	CAST IN PLACE WEIR STRUCTURE
18	2'X6' CATCH BASIN (DBL INLET)
19	2'X3' CATCH BASIN
20	36" FLARED END OUTFALL
21	CONCRETE PLUG PIPE
22	R&R TYPE A C&G
23	R&R 4" CONCRETE SIDEWALK
24	R&R VINYL FENCE
25	R&R WATERWAY
26	R&R SIDEWALK/FLATWORK (6" THICK)
27	ADA DETECTABLE SURFACE (GRAY)
28	SOD LANDSCAPE RESTORATION
30	8" WATERLINE LOOP
31	PROTECT IN PLACE

SCALE: HORIZONTAL 0 20 40

SD-4

0

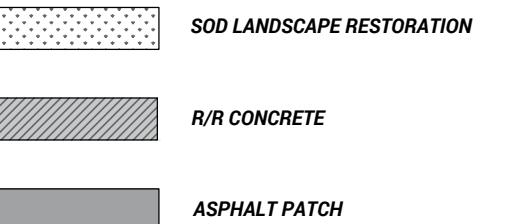
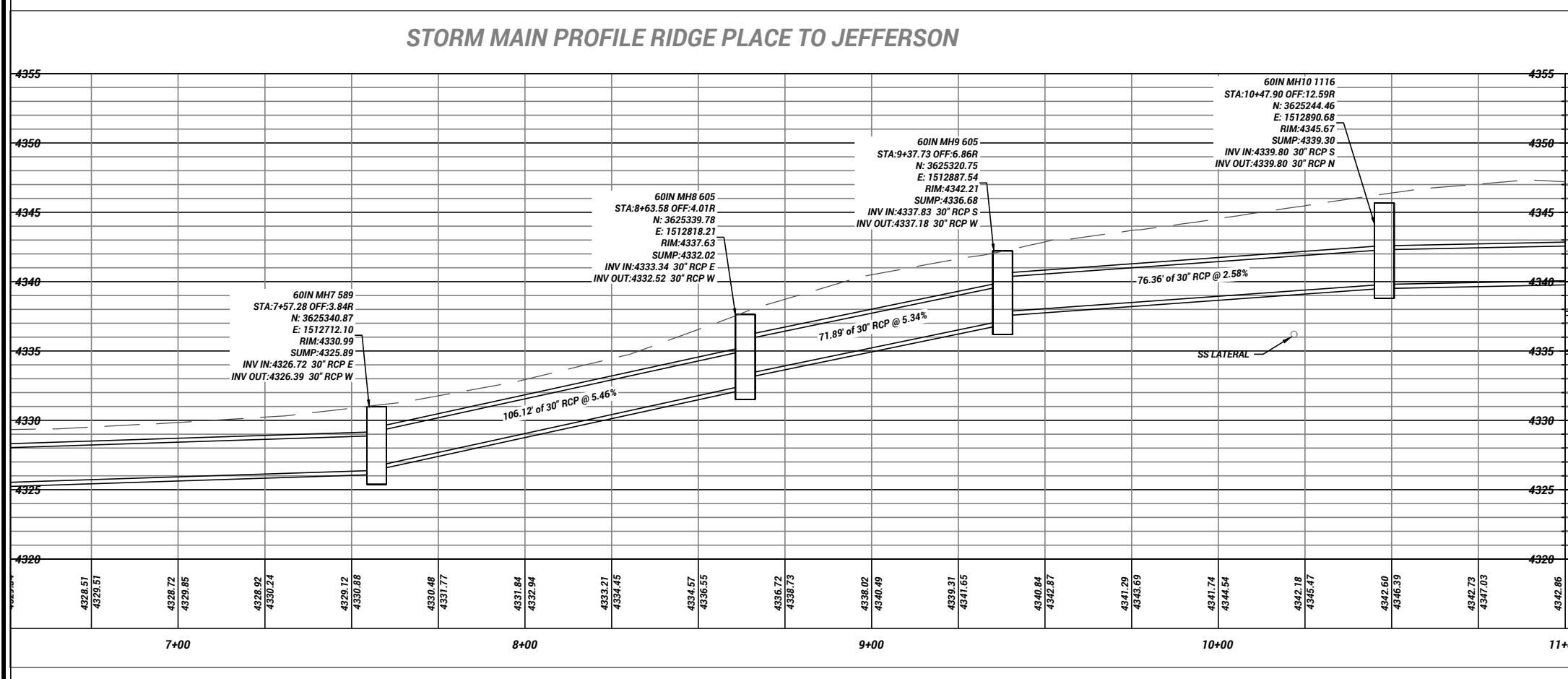
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REVISION

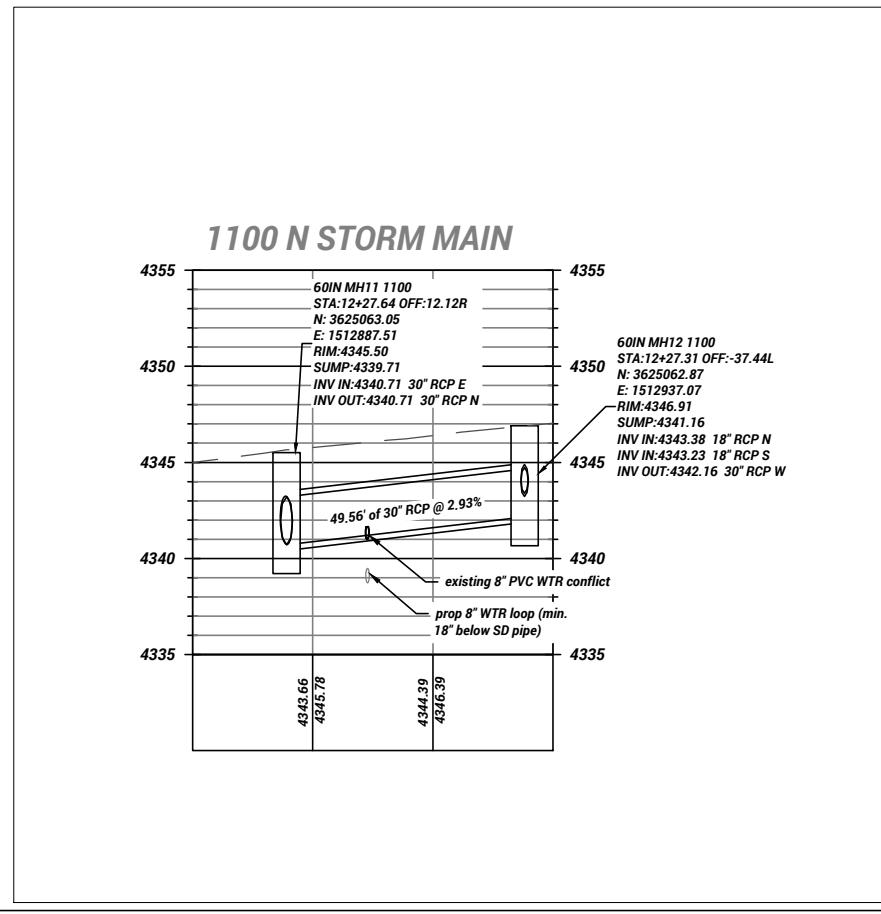
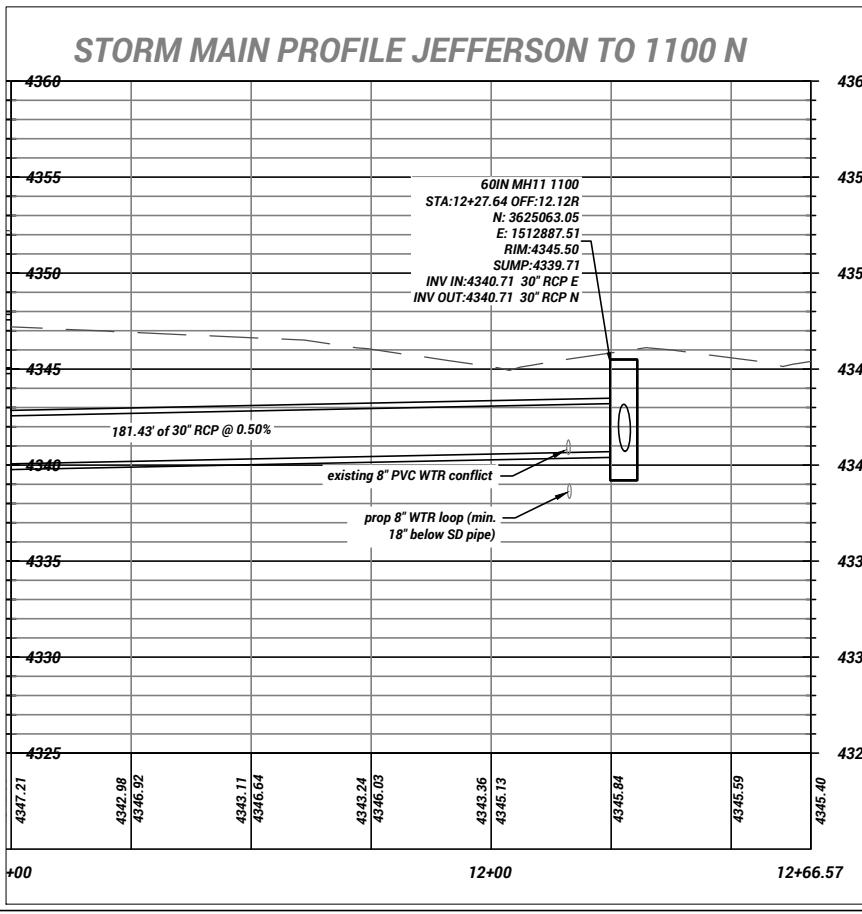
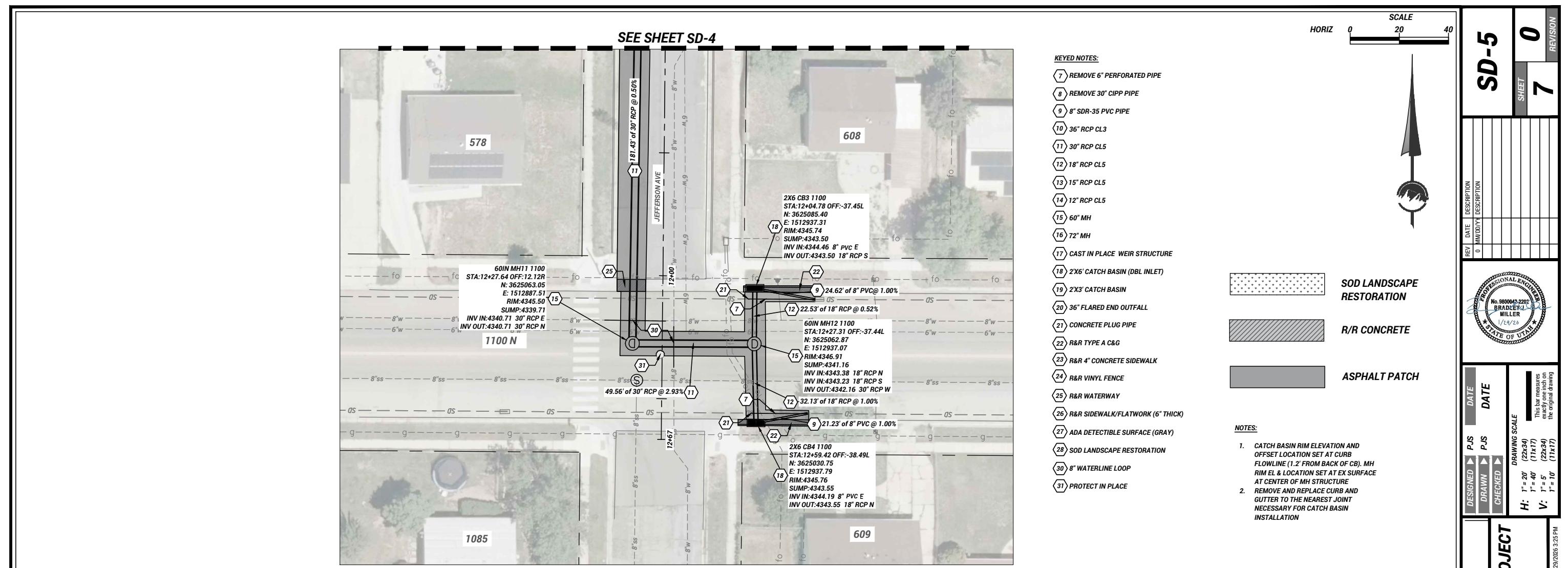
SEE SHEET SD-3

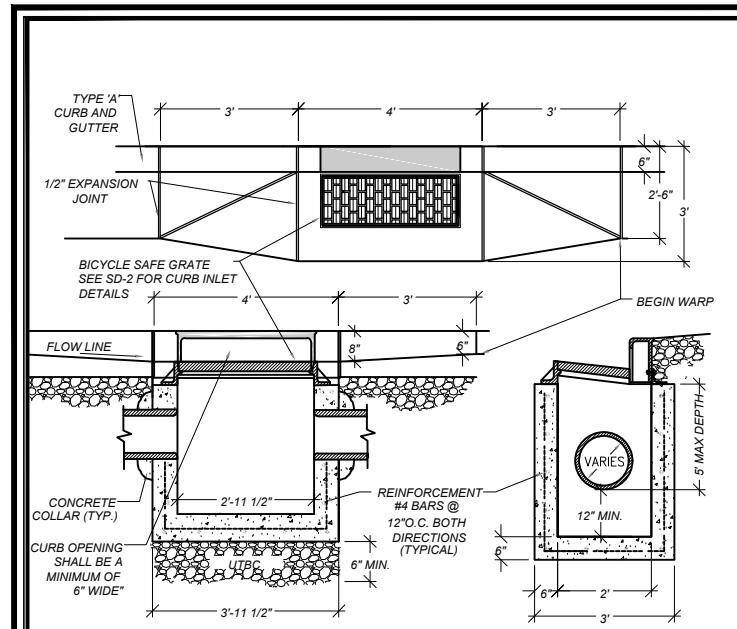
SEE SHEET SD-5



NOTES:

1. CATCH BASIN RIM ELEVATION AND OFFSET LOCATION SET AT CURB FLOWLINE (1.2' FROM BACK OF CB). MH RIM EL & LOCATION SET AT EX SURFACE AT CENTER OF MH STRUCTURE
2. REMOVE AND REPLACE CURB AND GUTTER TO THE NEAREST JOINT NECESSARY FOR CATCH BASIN INSTALLATION

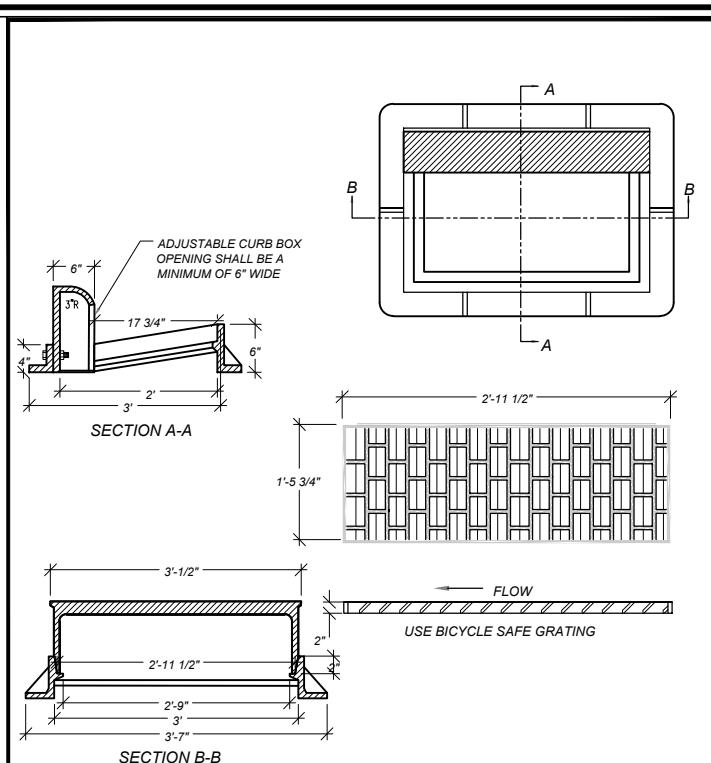




NOTES:

1. COMPACT BASE COURSE AND BACKFILL PER APWA SECTION 31 23 26 TO A DENSITY OF 95 PERCENT. MAXIMUM LIFT THICKNESS IS 8" BEFORE COMPACTION.
- 1.1. BACKFILL: PROVIDE AND PLACE PER APWA SECTION 31 23 23 ON ALL SIDES OF THE BASIN.
- 1.2. PROVIDE BASE COURSE MATERIAL PER APWA SECTION 32 11 23. PLACE MATERIAL PER APWA SECTION 31 23 23.
2. REINFORCEMENT SHALL BE PER ASTM A 615, GRADE 60, DEFORMED STEEL.
3. CONCRETE SHALL BE CLASS 4000 PER APWA SECTION 03 30 04. PLACE CONCRETE PER APWA SECTION 03 30 10. CURE PER APWA SECTION 03 39 00. PRECAST CATCH BASINS ARE ACCEPTABLE.
4. CURB FACE OPENING SHALL BE AT LEAST 6" WIDE. PROVIDE A 2" DROP BETWEEN THE 'BEGIN WARP' LINE IN THE GUTTER AND THE TOP OF THE GRATE AT THE CURB OPENING.

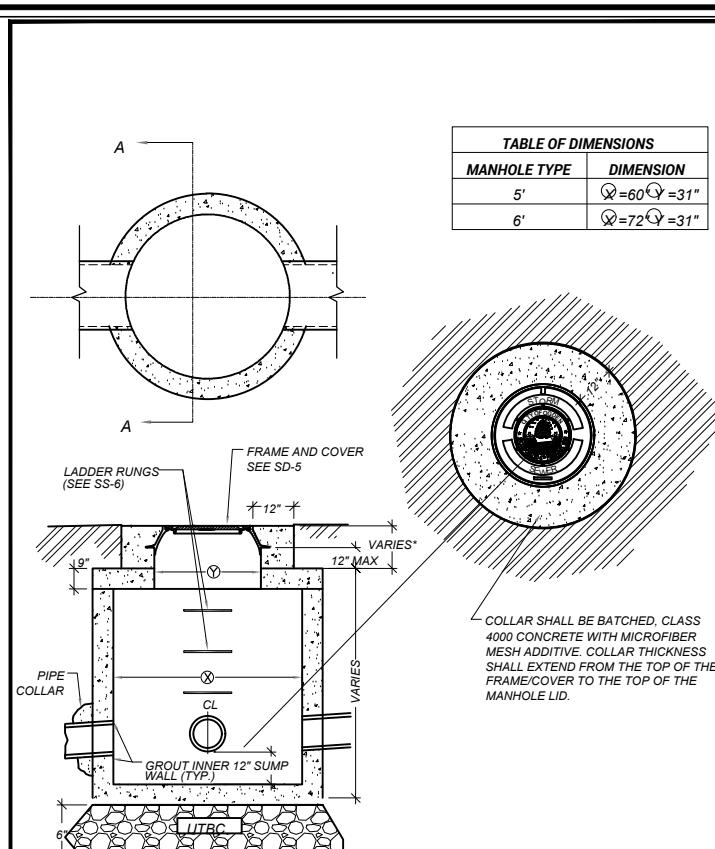
OGDEN CITY ENGINEERING - STANDARD DRAWINGS		
CATCH BASIN WITH CURB INLET		
SD-1		
TAYLOR NIELSEN, CITY ENGINEER	SHEET 1 OF 1	2025



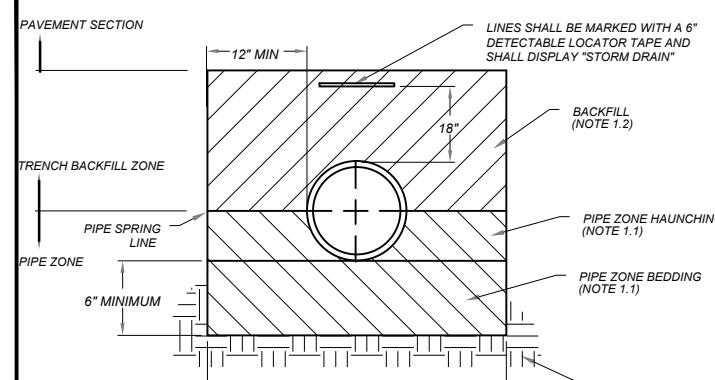
NOTES:

1. CASTING: GREY IRON CLASS 35 MINIMUM PER ASTM A 48.
2. COATINGS: EXCEPT MACHINED SURFACES, COAT ALL METAL PARTS WITH ASPHALTUM PAINT.
3. USE STAINLESS STEEL BOLTS, NUTS, AND WASHERS.
4. SEE SD-1 FOR INLET BOX DETAILS.

OGDEN CITY ENGINEERING - STANDARD DRAWINGS		
35 1/2" GRATE AND FRAME WITH ADJUSTABLE CURB BOX		
SD-2		
TAYLOR NIELSEN, CITY ENGINEER	SHEET 1 OF 1	2025



OGDEN CITY ENGINEERING - STANDARD DRAWINGS		
STORM DRAIN MANHOLE (PRECAST)		
SD-4		
TAYLOR NIELSEN, CITY ENGINEER	SHEET 1 OF 2	2025



NOTES:

1. DO NOT USE SEWER ROCK OR RECYCLED FILL MATERIAL IN THE PIPE ZONE WITHOUT THE APPROVAL OF THE CITY ENGINEER. IF MANUFACTURER RECOMMENDS ANYTHING CONTRARY TO WHAT IS LISTED BELOW, CONSULT WITH THE ENGINEERING DEPARTMENT.
- 1.1. HAUNCHING/BEDDING: AGGREGATE BASE COURSE (UTBC) PER 32 11 23.
 - 1.1.1. WATER JETTING IS NOT ALLOWED.
 - 1.1.2. SUBMISSION OF COMPACTION TEST DATA FOR THE HAUNCHING AREAS MAY BE REQUESTED AT ANY TIME.
- 1.2. BACKFILL (ABOVE THE PIPE ZONE): IMPORT STRUCTURAL FILL (3" MINUS PER APWA 31 05 13 OR UTBC PER 32 11 23).
- 1.2.1. COMPACT PER SECTION 31 23 26 TO A DENSITY OF 95 PERCENT OR GREATER. MAXIMUM LIFT THICKNESS BEFORE COMPACTION IS 8" WHEN USING RIDING AND 6" WHEN USING HAND COMPACTION EQUIPMENT.

2. PIPE ZONE WIDTH IS RECOMMENDED BY THE MANUFACTURER OF THE PIPE. WIDTH OF PIPE ZONE IS MEASURED AT THE PIPE SPRING LINE AND INCLUDES ANY NECESSARY SHEATHING. FOLLOW MANUFACTURE RECOMMENDATIONS FOR ANY TRENCH BOX APPLICATIONS.

3. INSTALL THE PIPE IN THE CENTER OF THE TRENCH. THE EDGE OF THE PIPE WALL SHALL BE A MINIMUM DISTANCE OF 12" FROM THE TRENCH WALL.

4. PEA GRAVEL (GRAVEL WITH NOMINAL SIZE LESS THAN 3/4") IS NOT ALLOWED IN ANY PART OF THE TRENCH.

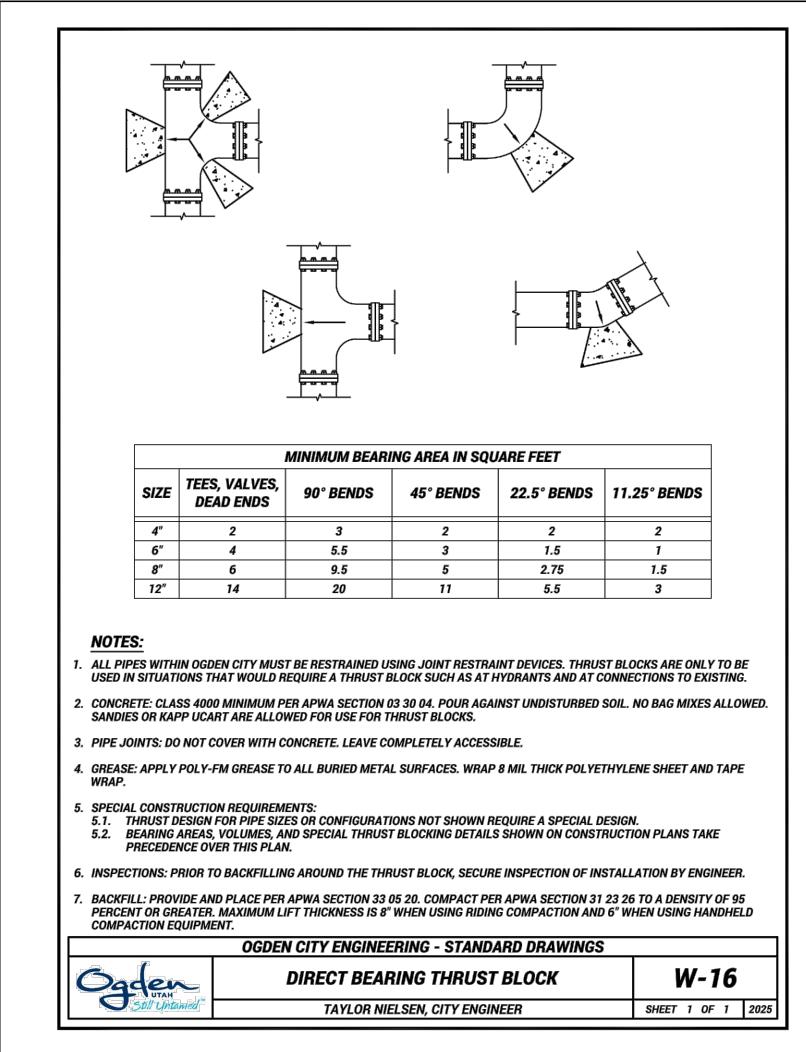
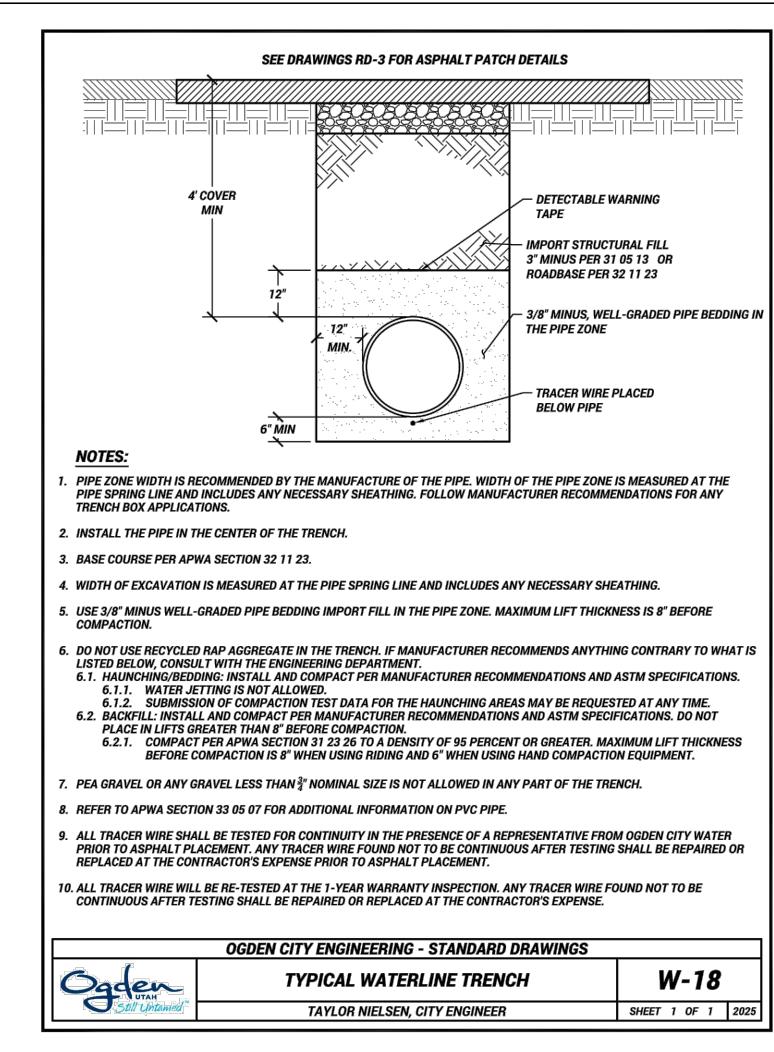
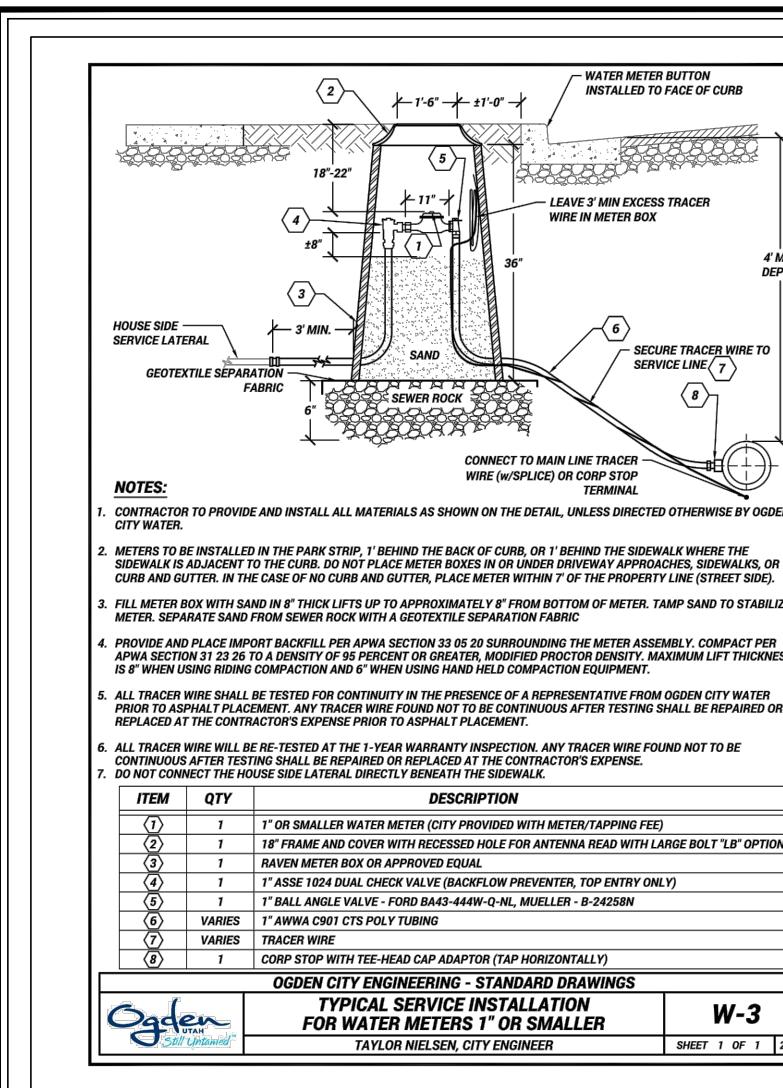
5. FOUNDATION STABILIZATION REQUIRES SEWER ROCK PER APWA SECTION 31 05 13.

5.1. INSTALLATION OF STABILIZATION-SEPARATION GEOTEXTILE PER APWA SECTION 31 05 19 WILL BE REQUIRED TO SEPARATE BACKFILL MATERIAL AND NATIVE SUBGRADE MATERIAL IF SEWER ROCK CANNOT PROVIDE A WORKING SURFACE OR PREVENT SOIL MIGRATION.

OGDEN CITY ENGINEERING - STANDARD DRAWINGS		
STORM DRAIN PIPE ZONE		
SD-6		
TAYLOR NIELSEN, CITY ENGINEER	SHEET 1 OF 1	2025

DT-1	
SHEET	8
REV	0
DATE	1/29/2026
DESCRIPTION	
0 MM/DD/YY	
PROFESSIONAL ENGINEER SIGNATURE	
No. 9800043-2025 BRADLEY MILLER STATE OF UTAH 1/29/26	
DRAWING SCALE	
P.J.S.	(22x34)
P.J.S.	(11x17)
This drawing is an electronic copy of the original drawing.	
H:	None
DRAWN	(22x34)
DATE	1/29/2026
V:	None
CHECKED	(22x34)
DESIGNED	(11x17)

STANDARD DRAWINGS	
DETENTION TO 1100 N STORM PROJECT	
DRAWING NAME:	G1 Title 1100 N.dwg
PLOT DATE:	1/29/2026 2:58 PM



DETENTION TO 1100 N STORM PROJECT

DETAILS

DT-2

0

REVISION

REV. 0	DATE 1/29/2026	DESCRIPTION
0 MM/0 DD/0 YY DESCRIPTION		
1/29/26		

PROFESSIONAL ENGINEER
No. 9800402-2026
BRADLEY E. MILLER
1/29/26
* STATE OF UTAH *

DESIGNED P.J.S.	DRAWN P.J.S.	DATE 1/29/2026
DRAWN	CHECKED	
H: NONE (22x34)	DRAWING SCALE	This drawing measures exactly one inch on the original drawing
V: NONE (22x34)		(11x17)

DETAILS

SEED MIX

MIX NAME: BARKLEY CABIN BLEND

%PURE	COMMON NAME
46.46%	FESCUE SHEEP
33.78%	CRESTED WHEATGRASS
14.80%	STREAMBANK WHEATGRASS
4.95%	INERT MATTER
0.01%	OTHER CROP
0.01%	WEED SEED

DRAWING NAME: G:\\Title 1100 N.dwg

PLOT DATE: 1/29/2026 2:58 PM

Ogden UTAH Still Untamed™

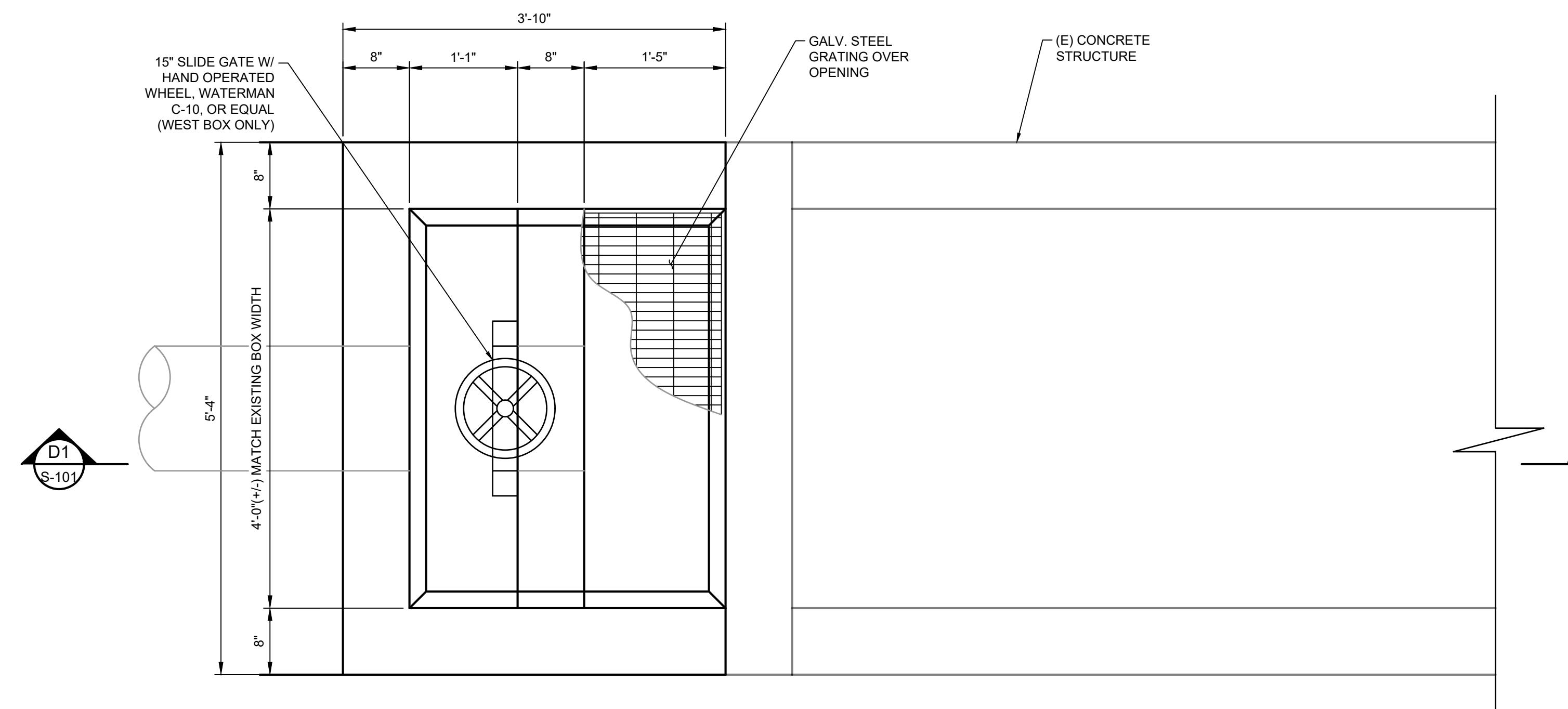
2549 Washington Blvd, Suite 760 Ogden, UT 84401
Phone: 801-629-8980 engineering.ogdenutah.com

GENERAL REQUIREMENTS		DESIGN CRITERIA	STRUCTURAL MATERIALS	FORMWORK	J-U-B ENGINEERS, INC. 745 W. Hanley Ave. Suite 301 Coeur d'Alene, ID 83815 Phone: 208.762.8787 www.jub.com	
<p>A. THESE GENERAL STRUCTURAL NOTES AND SPECIFICATIONS SUPPLEMENT THE PROJECT WRITTEN TECHNICAL SPECIFICATIONS AND THE PROJECT STRUCTURAL DRAWINGS.</p> <p>B. THE CONTRACTOR IS RESPONSIBLE FOR ALL CONSTRUCTION BRACING, TEMPORARY SHORING, AND OTHER SITE SAFETY CONTROLS REQUIRED DURING CONSTRUCTION IN ACCORDANCE WITH ALL APPLICABLE LOCAL, STATE AND FEDERAL REGULATIONS, TO ENSURE THE STABILITY AND SAFETY OF ALL CONSTRUCTION UNTIL IT IS COMPLETED AND SELF-SUPPORTING.</p> <p>C. THE CONTRACTOR IS RESPONSIBLE FOR ALL WATER, BOTH ABOVE AND BELOW GROUND, RUNOFF AND OTHER ENVIRONMENTAL CONTROLS REQUIRED DURING CONSTRUCTION TO ENSURE THE SITE IS MAINTAINED IN COMPLIANCE WITH ALL APPLICABLE LOCAL, STATE AND FEDERAL REGULATIONS.</p> <p>D. DETAILS ON THESE PLANS ARE INTENDED TO DEPICT THE GENERAL CONSTRUCTION DETAILS AND METHODS FOR THIS STRUCTURE. CONNECTION DETAILS AND CONDITIONS NOT SPECIFICALLY SHOWN THAT ARE SIMILAR IN NATURE TO THOSE THAT ARE SPECIFIED SHALL BE ASSUMED ONE AND THE SAME. IF QUESTIONS REGARDING THE APPLICATION OF DETAILS ARE ENCOUNTERED, NOTIFY THE ARCHITECT/ENGINEER FOR CLARIFICATION OR INSTRUCTION.</p> <p>E. PRIOR TO IMPLEMENTING ANY CHANGES TO THESE PLANS, THE ARCHITECT/ENGINEER SHALL BE NOTIFIED IN WRITING FOR THEIR WRITTEN APPROVAL. CHANGES IMPLEMENTED WITHOUT THE ARCHITECT/ENGINEER WRITTEN APPROVAL SHALL RELIEVE THE ARCHITECT/ENGINEER OF ANY CLAIM OR LIABILITY RESULTING FROM THAT PORTION OF THE STRUCTURE CHANGED OR AFFECTED BY THE CHANGE.</p>		<p>A. OCCUPANCY OR USE: IBC TABLE 1607.1: UTILITY</p> <ul style="list-style-type: none"> i RISK CATEGORY: ASCE-7 TABLE 1.5-2: II <p>B. LIVE LOADS:</p> <ul style="list-style-type: none"> i ROOF LIVE LOAD: 20 PSF <p>C. SOIL DESIGN PARAMETERS (ASSUMED):</p> <ul style="list-style-type: none"> ii NET ALLOWABLE SOIL BEARING PRESSURES; $Q_n = 1500 \text{ PSF}$ iii iv EQUIVALENT DRAINED FLUID PRESSURES (ABOVE GW) <ul style="list-style-type: none"> (a) ACTIVE; $K_a = 35 \text{ PCF}$ (b) AT REST; $K_r = 55 \text{ PCF}$ (c) PASSIVE; $K_p = 200 \text{ PCF}$ v COEFFICIENT OF FRICTION, SOIL TO CONCRETE; $F = 0.25$ vii FROST DEPTH; $DF = 2'-6"$ 	<p>A. STRUCTURAL BOLTS: HIGH STRENGTH BOLTS SHALL BE ASTM F3125 GRADE A325. NUTS FOR HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM A563, GRADE DH, HEAVY HEX.</p> <p>B. ANCHOR RODS: ANCHOR RODS SET INTO CONCRETE SHALL BE ASTM F1554, FY=55 KSI. NUTS FOR ANCHOR RODS SHALL CONFORM TO ASTM A563, GRADE A, HEAVY HEX.</p> <p>C. THREADED STEEL RODS: THREADED STEEL RODS SHALL CONFORM TO ASTM A36, FY=36 KSI. NUTS FOR THREADED RODS SHALL CONFORM TO ASTM A563, GRADE A, HEAVY HEX.</p> <p>D. WASHERS: ALL WASHERS SHALL CONFORM TO ASTM F436.</p> <p>E. BOLT PLACEMENT: ALL BOLTS SHALL BE ON MEMBER STANDARD GAGE LINES EXCEPT AS NOTED OTHERWISE.</p> <p>F. STAINLESS STEEL:</p> <ul style="list-style-type: none"> i ALL STEEL NOTED AS STAINLESS INCLUDING SHAPES, PLATES, BARS, PIPE, TUBING, FASTENERS, ANCHOR RODS AND ASSOCIATED MISCELLANEOUS STEEL ITEMS SHALL BE STAINLESS STEEL, GRADE 304 OR 316, UNLESS SPECIFICALLY NOTED OTHERWISE. ii STRUCTURAL SHAPES, PLATES & BARS: ASTM A276 WITH FY = 30 KSI. iii STRUCTURAL STEEL TUBING: ROUND, SQUARE OR RECTANGULAR STRUCTURAL STEEL TUBING SHALL BE ASTM A269 WITH FY = 35 KSI. iv STRUCTURAL BOLTS: ASTM A593 GRADE G OR H, MINIMUM FY = 65 KSI. v NUTS: ASTM A594 GRADE C OR D, HEAVY HEX NUTS. vi WASHERS: STAINLESS STEEL GRADE 304 OR 316, MINIMUM FY = 30 KSI. vii ANCHOR RODS (SET IN CONCRETE): ASTM A593 GRADE 304, MIN. FY = 30 KSI. viii THREADED BRACING RODS: ASTM A193 GRADE B8, MINIMUM FY=65 KSI ix PIPE: ASTM A312, MINIMUM FY = 30 KSI. x ROUND OR SQUARE TUBING: TUBING SHALL CONFORM TO ASTM A269, MINIMUM FY = 35 KSI. <p>G. PROJECT CONCRETE MIX TYPES: CONCRETE SHALL BE PROPORTIONED AND FURNISHED FOR THE VARIOUS PROJECT USES AS INDICATED ON THE PLANS AND AS FOLLOWS:</p> <ul style="list-style-type: none"> i M2500-SEC: SECONDARY CONCRETE MIX FOR FILL AND BACKFILL AROUND BURIED PIPES UNDERNEATH STRUCTURAL FOOTINGS AND FOUNDATION SLABS: FC = 2,500 PSI, ABSOLUTE WATER-CEMENT RATIO BY WEIGHT = 0.55, AIR CONTENT = 6% (+/- 1.5%), MAXIMUM AGGREGATE SIZE 3/8-INCH; SLUMP 8-INCH. vi M4500-SPS: SUPER-PLASTICIZED CONCRETE MIX FOR LIQUID-RETAINING CONCRETE STRUCTURAL WALLS AND FOUNDATION SLABS: FC = 4,500 PSI, ABSOLUTE WATER-CEMENT RATIO BY WEIGHT = 0.40, AIR CONTENT = 6% (+/- 1.5%). x M-SLURRY: SLURRY CONCRETE FOR HORIZONTAL CONSTRUCTION JOINTS IN WALLS. A FLOWABLE MIX CONSISTING OF SAND, 3/8-INCH MAXIMUM AGGREGATE, WATER AND A MINIMUM 1,150 POUNDS OF CEMENT PER CUBIC YARD. xi M-CDF: MIX FOR CONTROLLED DENSITY FILL (CDF) OR CONTROLLED LOW STRENGTH MATERIAL (CLSM). CDF SHALL BE A MIXTURE OF CEMENT, FINE AND COARSE AGGREGATE, FLY ASH AND ADMIXTURES FORMULATED TO BE FLOWABLE AND SELF-CONSOLIDATING WITH A NET 28 DAY COMPRESSIVE STRENGTH OF 200 TO 300 PSI. ii CONCRETE MIX COMPONENTS: iii A WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494, USED IN STRICT CONFORMANCE WITH THE MANUFACTURERS INSTRUCTIONS, SHALL BE INCORPORATED IN ALL CONCRETE MIX DESIGNS. iv FOR ALL WATER-RETAINING CONCRETE STRUCTURAL WALLS AND SLABS, A HIGH-RANGE WATER-REDUCING (HRWR) ADMIXTURE CONFORMING TO ASTM C494, TYPE F OR G, SHALL BE USED. THE TOTAL SLUMP SHALL BE LESS THAN 10-IN. viii HIGHER WATER-CEMENT RATIOS THAN SHOWN ABOVE MAY BE USED IF SUBSTANTIATED IN ACCORDANCE WITH ACI 318. ix FLY-ASH CONFORMING TO ASTM C618 TYPE F OR C, MAY REPLACE UP TO 20% OF THE CEMENT CONTENT, PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA. xv CEMENT: ASTM C150 TYPE II / ASTM C595 IL (10)MS. xvi CEMENT: ASTM C845 TYPE K FOR SHRINKAGE COMPENSATING MIXES. xvii WATER: CLEAN & POTABLE. xviii AIR ENTRAINING AGENT: ASTM C260. EXCEPT WHERE NOTED NON-AIR ENTRAINED. xix AGGREGATE: 0.75-INCH MAXIMUM AGGREGATE PER ASTM C33. UNLESS NOTED OTHERWISE. xx MIX PROPORTIONING: ACI 211.1 AND 350. i CONCRETE ACCESSORIES: ii REINFORCING STEEL: REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60; #3 BARS MAY BE GRADE 40. iii REINFORCING STEEL TO BE WELDED: ALL REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706 GRADE 60, LOW-ALLOY, DEFORMED REINFORCING STEEL. iv WELDED WIRE FABRIC: ASTM A185 OR A497. v STAINLESS STEEL BARS: STAINLESS STEEL BARS SHALL CONFORM TO ASTM A855. vi WIRE: PLAIN WIRE SHALL CONFORM TO ASTM A 82. DEFORMED WIRE SHALL CONFORM TO ASTM A496. vii JOINTING MATERIALS: IN ACCORDANCE WITH ACI 352, ALL JOINTING MATERIALS INCLUDING WATER-STOPS, EXPANSION JOINTS AND SEALANTS SHALL BE RESISTANT TO CHEMICAL ATTACK FOR THE DESIGN LIFE OF THE FACILITY. SEALANTS SHALL CONFORM TO ASTM C920 AND FEDERAL SPECIFICATION TT-S-00227E AND PVC WATER-STOP SHALL CONFORM TO ASTM D570, ASTM D746, ASTM D1149 AND CRD-C572. viii NON-SHRINK GROUT: ALL NON-SHRINK GROUT NOTED ON THE PLANS SHALL BE NON-SHRINK, NON-METALLIC GROUT WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 7,000 PSI. ix EXPANSION BOLTS: BOLTS NOTED ON THE PLANS AS EXPANSION BOLTS SHALL BE HILTI KWIK BOLT 3, STUD ANCHORS; SIZE AND EMBEDMENT AS NOTED ON THE DRAWINGS, INSTALLED PER THE MANUFACTURERS RECOMMENDATIONS; OR AN APPROVED EQUAL. 	<p>A. FORMS SHALL RESULT IN A FINAL STRUCTURE THAT CONFORMS TO SHAPES, LINES, AND DIMENSIONS OF THE MEMBERS AS REQUIRED BY THE DESIGN DRAWINGS AND SPECIFICATIONS.</p> <ul style="list-style-type: none"> i DESIGN OF FORMWORK SHALL BE IN ACCORDANCE WITH ACI 318/350. ii FORMWORK SHALL BE IN ACCORDANCE WITH ACI 347, GUIDE TO FORMWORK FOR CONCRETE. <p>B. SPECIAL FORMWORK DESIGN FORMWORK AS LISTED IN THIS SECTION SHALL BE DESIGNED BY A PROFESSIONAL STRUCTURAL ENGINEER CURRENTLY REGISTERED IN THE STATE OF UTAH. DESIGN CALCULATIONS AND FORMWORK PLANS AND DETAILS, SEALED AND SIGNED BY THE FORMWORK ENGINEER OF RECORD SHALL BE SUBMITTED UNDER THE DEFERRED SUBMITTAL SECTION OF THE PROJECT SPECIFICATIONS. SUBMIT SPECIAL FORMWORK DESIGNS FOR THE FOLLOWING ITEMS:</p> <p>C. TOLERANCES FOR FINISHED CONCRETE SURFACES SHALL MEET THE FOLLOWING REQUIREMENTS, CLASS OF SURFACE IS PER ACI 347:</p> <ul style="list-style-type: none"> i FOOTINGS: CLASS C ii FOUNDATION WALLS: CLASS B iii ABOVE GRADE CONCRETE NOT VISIBLE TO SIGHT: CLASS B iv ABOVE-GRADE CONCRETE VISIBLE TO SIGHT: CLASS A <p>D. REMOVAL OF FORMS:</p> <ul style="list-style-type: none"> i CONCRETE FORMS SHALL NOT BE REMOVED UNTIL THE RETAINED CONCRETE HAS REACHED THE FOLLOWING MINIMUM PERCENTAGE OF THE REQUIRED 28 DAY COMPRESSIVE STRENGTH: (a) FOOTINGS AND BASE SLABS ON GRADE: 50% OF F'c. (b) FOUNDATION WALLS AND COLUMNS: 67% OF F'c. (c) ELEVATED STRUCTURAL SLABS, BEAMS AND JOISTS: 95% F'c. ii WHERE CONCRETE CYLINDER TESTS ARE NOT AVAILABLE FOR STRENGTH VERIFICATION THE FOLLOWING GUIDE MAY BE USED WHEN PERMITTED BY THE PROJECT ENGINEER: (a) FOOTINGS AND BASE SLABS ON GRADE: 12 HOURS. (b) FOUNDATION WALLS AND COLUMNS: 24 HOURS. (c) ELEVATED STRUCTURAL SLABS, BEAMS AND JOISTS: <ul style="list-style-type: none"> • SPANS UNDER 10-FEET: 4 DAYS • SPANS BETWEEN 10-FEET AND 15-FEET: 7 DAYS. • SPANS BETWEEN 15-FEET AND 20-FEET: 10 DAYS. • SPANS GREATER THAN 20-FEET: BY CYLINDER STRENGTH VERIFICATION ONLY. <p>E. EMBEDMENTS IN CONCRETE:</p> <ul style="list-style-type: none"> i CONDUITS, PIPES, AND SLEEVES OF ANY MATERIAL NOT HARMFUL TO CONCRETE AND WITHIN LIMITATIONS OF ACI 318/350 SHALL BE PERMITTED TO BE EMBEDDED IN CONCRETE WITH APPROVAL OF THE PROJECT ENGINEER, PROVIDED THEY ARE NOT CONSIDERED TO REPLACE STRUCTURALLY THE DISPLACED CONCRETE, EXCEPT AS PROVIDED IN ACI 350. ii CONDUITS AND PIPES OF ALUMINUM SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE UNLESS EFFECTIVELY COATED OR COVERED TO PREVENT ALUMINUM-CONCRETE REACTION OR ELECTROLYTIC ACTION BETWEEN ALUMINUM AND STEEL. <p>F. CONSTRUCTION JOINTS:</p> <ul style="list-style-type: none"> i CONSTRUCTION JOINTS SHALL ONLY BE PLACED WHERE INDICATED ON THE PROJECT DRAWINGS OR AS APPROVED BY THE PROJECT ENGINEER. ii CONSTRUCTION JOINTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 318/350. 	CONCRETE FINISHING	
<p>A. INTERNATIONAL BUILDING CODE, IBC 2021 EDITION.</p> <p>B. MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE 7-16.</p> <p>C. AMERICAN CONCRETE INSTITUTE, ACI 318, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE; CURRENT EDITION.</p> <p>D. AMERICAN CONCRETE INSTITUTE, ACI 350, CODE REQUIREMENTS FOR ENVIRONMENTAL ENGINEERING CONCRETE STRUCTURES; CURRENT EDITION.</p> <p>E. AMERICAN CONCRETE INSTITUTE, ACI 301, SPECIFICATIONS FOR STRUCTURAL CONCRETE.</p> <p>F. AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 15TH EDITION, STEEL CONSTRUCTION MANUAL.</p> <p>G. AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 360, SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS; CURRENT EDITION.</p> <p>H. AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 341, SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS; CURRENT EDITION.</p> <p>I. AMERICAN WELDING SOCIETY, AWS D1.1 CURRENT EDITION, STRUCTURAL WELDING CODE.</p>		<p>A. FORMED CONCRETE SURFACES. AFTER REMOVAL OF FORMS, GIVE EACH FORMED SURFACE ONE OR MORE OF THE FOLLOWING FINISHES IN CONFORMANCE WITH ACI 301 FINISHING FORMED SURFACES.</p> <ul style="list-style-type: none"> i CONCRETE STRUCTURES: (a) CONCRETE FOOTINGS AND FOUNDATIONS NOT EXPOSED TO VIEW. PROVIDE AN AS-CAST FINISH. (b) FOUNDATION WALL AND OTHER SURFACES BELOW GRADE AND NOT EXPOSED TO VIEW. PROVIDE A SURFACE FINISH TYPE SF-1.0. (c) INTERIOR, EXTERIOR AND TOP SURFACES EXPOSED TO VIEW TO 6-INCHES BELOW GRADE. PROVIDE A SURFACE FINISH TYPE SF-2.0. (d) COLUMN, BEAM AND JOIST SURFACES THAT ARE EXPOSED TO VIEW. PROVIDE A SURFACE FINISH TYPE SF-2.0. (e) INTERIOR CONCRETE SURFACES TO BE PAINTED OR RECEIVE OTHER COATING SYSTEMS SHALL RECEIVE A SURFACE FINISH TYPE SF-3.0. ii LIQUID RETAINING CONCRETE STRUCTURES: (a) INTERIOR SURFACES FROM TOP OF WALL TO FLOOR SLAB, EXTERIOR AND TOP SURFACES EXPOSED TO VIEW TO 6-INCHES BELOW GRADE. PROVIDE A GROUT-CLEANED FINISH. PRESSURE-WASH ALL CONCRETE SURFACES TO RECEIVE A GROUT-CLEANED FINISH WITH HIGH-PRESSURE WATER PRIOR TO GROUTING. (ALTERNATE, PROVIDE A SURFACE FINISH TYPE SF-3.0.) (b) SURFACES BELOW GRADE AND NOT EXPOSED TO VIEW. PROVIDE A SURFACE FINISH TYPE SF-2.0. iii SPECIAL OR ARCHITECTURAL FINISHES: REFER TO THE ARCHITECTURAL SPECIFICATIONS FOR SPECIAL OR ARCHITECTURAL FINISH REQUIREMENTS. <p>B. UNFORMED CONCRETE SURFACES. UNFORMED CONCRETE SURFACES INCLUDING THE TOP SURFACE OF ALL CONCRETE ROOF AND FLOOR SLABS SHALL BE FINISHED IN ACCORDANCE WITH ACI 301 AND ACI 302.</p> <ul style="list-style-type: none"> (a) FOR THE TOP SURFACES OF WALLS, PROVIDE A SCRATCHED FINISH. (b) INTERIOR OFFICES, LABORATORY SPACES AND OTHER AREAS RECEIVING ONLY LIGHT FOOT TRAFFIC SHALL RECEIVE A TROWELED FINISH. (c) INTERIOR GARAGE, INDUSTRIAL OR WORK AREAS SUBJECT TO EQUIPMENT OR TRAFFIC LOADS SHALL RECEIVE A BROOM FINISH. (d) PROVIDE A NONSLIP FINISH FOR EXTERIOR SURFACES AND WHERE INDICATED ON THE PLANS. (e) REFER TO THE ARCHITECTURAL PLANS FOR FINISH REQUIREMENTS FOR FLOORS TO RECEIVE ARCHITECTURAL COVERINGS. <p>C. SAWED CONTRACTION JOINTS. CONFORM TO ACI 301.</p>	GENERAL STRUCTURAL NOTES			
<p>SUBMITTALS</p> <p>A. SUBMIT PRODUCT OR MATERIAL INFORMATION TO THE ARCHITECT/ENGINEER FOR REVIEW FOR THE FOLLOWING ITEMS:</p> <ul style="list-style-type: none"> i CONCRETE MIX DESIGNS AND ADMIXTURES. ii NON-SHRINK GROUT. iii EXPANSION BOLTS. iv ADHESIVE ANCHORS. 					JACKSON CANYON VIEW SD ANALYSIS OGDEN CITY	
<p>SHOP DRAWINGS</p> <p>A. SUBMIT SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION OF THE FOLLOWING ITEMS:</p> <ul style="list-style-type: none"> i REINFORCING STEEL FOR ALL CONCRETE. ii MISCELLANEOUS STEEL FABRICATIONS INCLUDING BAR-GRATING 						
<p>FOUNDATIONS</p> <p>A. ALL FOOTINGS TO BE PLACED ON FIRM UNDISTURBED, INORGANIC MATERIAL. PROOF ROLL SUB-GRADE PRIOR TO PLACING CONCRETE WHERE THE MATERIAL HAS BEEN DISTURBED BY THE EXCAVATING EQUIPMENT.</p> <p>B. ALL PIERS AND FOOTINGS OUTSIDE OR AT THE PERIMETER OF THE STRUCTURE, OR IN OTHER UNHEATED AREAS SHALL BE SET TO A DEPTH OF AT LEAST 24-IN. BELOW FINISH GRADE, UNLESS OTHERWISE NOTED ON THE PLANS.</p> <p>C. ALL FOUNDATIONS AND RETAINING WALLS BELOW FINISH GRADE SHALL RECEIVE AN APPROVED DAMP-PROOF COATING. FOUNDATION WALLS BELOW MAXIMUM ANTICIPATED GROUND WATER LEVELS SHALL RECEIVE AN APPROVED WATER-PROOF COATING; EXTEND WATER-PROOFING TO A MINIMUM OF 1'-0" ABOVE THE MAXIMUM ANTICIPATED GROUND WATER LEVEL.</p> <p>D. ALLOWABLE BEARING PRESSURE FOR ALL FOOTINGS $Q_a = 1500 \text{ PSF}$</p> <p>E. LOCAL AREAS OF SOFT AND/OR UNACCEPTABLE MATERIAL ENCOUNTERED AT BOTTOM OF FOOTING ELEVATIONS NOTED ON THE PLANS MUST BE OVER-EXCAVATED AND BROUGHT UP TO DESIGN GRADE WITH COMPACTED STRUCTURAL FILL OR LEAN CONCRETE FILL.</p> <p>F. ALL STRUCTURAL FILL AND/OR BACKFILL SHALL BE GRANULAR, FREE DRAINING, MATERIAL; UNIFIED SOILS CLASSIFICATION GW, GP, GM, OR SW; MAXIMUM AGGREGATE SIZE OF 3-IN AND NO MORE THAN 7% PASSING A NUMBER 200 SIEVE. MATERIAL SHALL BE PLACED IN LIFTS NO GREATER THAN 6-IN. IN DEPTH AND COMPACTED TO 95% OF MAXIMUM DENSITY AS DETERMINED PER ASTM D1557.</p> <p>G. DESIGN FOR THE MITIGATION OF SUBSURFACE WATER FLOW AND/OR PERCHED WATER TABLES SHALL BE THE RESPONSIBILITY OF OTHERS.</p> <p>H. AS INDICATED ON THE PLANS ALL EXCAVATED AREAS UNDERNEATH CONCRETE STRUCTURES SHALL BE BACKFILLED WITH CONTROLLED DENSITY FILL (CDF) OTHERWISE CALLED CONTROLLED LOW STRENGTH MATERIAL (CLSM). CDF SHALL BE A MIXTURE OF CEMENT, FINE AND COARSE AGGREGATE, FLY ASH AND ADMIXTURES FORMULATED TO BE FLOWABLE AND SELF-CONSOLIDATING WITH A NET 28 DAY COMPRESSIVE STRENGTH OF 200 TO 300 PSI.</p> <p>I. THE ENGINEER SHALL BE NOTIFIED IN WRITING IF ANY GROUND WATER, CLAY TYPE SOILS, DEBRIS OR UNCONSOLIDATED MATERIALS ARE ENCOUNTERED DURING EXCAVATIONS FOR FOUNDATIONS.</p>						
<p>WATER-RETAINING CONCRETE STRUCTURES</p> <p>A. CONCRETE TANKS, VAULTS, WELLS AND OTHER STRUCTURES INTENDED TO RETAIN AND HOLD WATER OR OTHER LIQUIDS SHALL BE WATER-TIGHT STRUCTURES. THE WATER-RESISTING WALLS AND FLOOR SLABS SHALL BE OF MONOLITHIC CONCRETE CONSTRUCTION WITH WATER-TIGHT JOINTS, CONSTRUCTED AS INDICATED ON THE PLANS OR AS DIRECTED BY THE PROJECT ENGINEER. WATER-RESISTING WALLS AND FLOORS SHALL BE UNIFORM IN FINISHED CONSTRUCTION FREE OF SPALLS, POCKETS, BLEMISHES AND OR CRACKS THAT MAY WEEP OR LEAK.</p> <p>B. CRACKS FOUND IN WATER-RESISTING WALLS, FLOORS AND/OR FOUNDATION SLABS THAT MAY WEEP OR LEAK SHALL BE REPAIRED AND/OR SEALED PER THE PROJECT SPECIFICATIONS, NOTES OR AS APPROVED BY THE PROJECT ENGINEER.</p>						
<p>GENERAL STRUCTURAL NOTES</p> <p>FILE #: 55-25-020, S-001X JUB PROJ #: 55-25-020 DRAWN BY: EM DESIGN BY: JCH CHECKED BY: --- ONE INCH AT FULL SIZE, IF NOT ONE INCH, SCALE ACCORDINGLY LAST UPDATED: 1/28/2026 SHEET NUMBER: S-001</p>						

CONCRETE QUALITY AND DETAILS		DETAILS OF REINFORCEMENT	 J-U-B ENGINEERS, INC. 745 W. Hanley Ave. Suite 301 Coeur d'Alene, ID 83815 Phone: 208.762.8787 www.jub.com				
<p>A. GENERAL. CONCRETE SHALL BE PROPORTIONED TO PROVIDE AN AVERAGE COMPRESSIVE STRENGTH, FC, AS PRESCRIBED IN ACI 318/350 AND SHALL SATISFY THE DURABILITY CRITERIA OF ACI 318/350.</p> <p>B. CONCRETE PROPORTIONS.</p> <ul style="list-style-type: none"> i CONCRETE MIX PROPORTIONING SHALL BE IN ACCORDANCE WITH ACI 211.1; STANDARD PRACTICE FOR SELECTING PROPORTIONS FOR NORMAL-DENSITY AND HIGH-DENSITY CONCRETE. ii CONCRETE MIX PROPORTIONING FOR LIGHTWEIGHT CONCRETE SHALL BE IN ACCORDANCE WITH ACI 211.2; STANDARD PRACTICE FOR SELECTING PROPORTIONS FOR LIGHTWEIGHT CONCRETE. <p>C. CONCRETE MIX VERIFICATION. CONCRETE MIX DESIGNS SHALL BE VERIFIED BY STANDARD 28-DAY CYLINDER TESTS PER ASTM C39.</p> <p>D. EVALUATION AND ACCEPTANCE OF CONCRETE. CONCRETE SHALL BE TESTED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318/350.</p> <p>E. MIXING & PLACING CONCRETE. CONCRETE SHALL BE PREPARED, MIXED, PLACED AND CONSOLIDATED IN ACCORDANCE WITH ACI 318/350 AND AS FOLLOWS:</p> <ul style="list-style-type: none"> i ACI 304: GUIDE FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE. ii ACI 309: GUIDE FOR CONSOLIDATION OF CONCRETE. <p>F. MINIMUM TIME BETWEEN ADJACENT PLACEMENTS:</p> <ul style="list-style-type: none"> i NON-LIQUID RETAINING STRUCTURES: <ul style="list-style-type: none"> (a) CONSTRUCTION JOINTS: FIVE (5) DAYS WET CURE, OR SEVEN (7) DAYS DRY CURE. (b) CONTROL JOINTS: TWO (2) DAYS. (c) EXPANSION JOINTS: ONE (1) DAY. ii FLOOR SLABS: <ul style="list-style-type: none"> (a) CONSTRUCTION JOINTS: SEVEN (7) DAYS WET CURE, OR TEN (10) DAYS DRY CURE. (b) CONTROL JOINTS: FOUR (4) DAYS. (c) EXPANSION JOINTS: ONE (1) DAY. iii LIQUID RETAINING STRUCTURES: <ul style="list-style-type: none"> (a) CONSTRUCTION JOINTS: TEN (10) DAYS WET CURE, OR FOURTEEN (14) DAYS DRY CURE. (b) CONTROL JOINTS: SIX (6) DAYS. <p>G. HORIZONTAL JOINTS IN REINFORCED CONCRETE WALLS:</p> <ul style="list-style-type: none"> i PROVIDE A LAYER OF SLURRY CONCRETE IN THE BOTTOM OF ALL HORIZONTAL WALL JOINTS, 1-INCH MINIMUM TO 2-INCH MAXIMUM THICKNESS. THOROUGHLY VIBRATE TO MIX CONCRETE & SLURRY TOGETHER. ii UNLESS OTHERWISE SPECIFICALLY DIRECTED, IN ALL HORIZONTAL CONSTRUCTION JOINTS IN NEW CONCRETE CONSTRUCTION CONTAINING WATER-STOPS, PROVIDE AN INITIAL LAYER OF SLURRY MIX TO AIR IN CONSOLIDATION OF FRESHLY PLACED CONCRETE AT THE JOINT INTERFACE. iii SLURRY MIX SHALL BE A MIXTURE OF 3/8-INCH MAXIMUM AGGREGATE, SAND, CEMENT AND WATER. iv PLACE SLURRY MIX 2-INCH MINIMUM TO 4-INCH MAXIMUM IN THICKNESS. v PLACE CONCRETE OVER SLURRY MIX WHILE SLURRY MIX IS STILL FLOWABLE. vi LIMIT INITIAL CONCRETE PLACEMENT ON TOP OF SLURRY MIX TO 12-INCHES IN THICKNESS. THOROUGHLY VIBRATE AND CONSOLIDATE CONCRETE AND SLURRY MIX TOGETHER. vii CONCRETE CURING. CONCRETE SHALL BE MAINTAINED ABOVE 50-DEGREES F AND IN A MOIST CONDITION FOR AT LEAST 7 DAYS AFTER PLACEMENT, EXCEPT WHEN CURED IN ACCORDANCE WITH ACI 308 GUIDE TO EXTERNAL CURING OF CONCRETE. <p>I. COLD WEATHER REQUIREMENTS. ADEQUATE EQUIPMENT SHALL BE PROVIDED FOR HEATING CONCRETE MATERIALS AND PROTECTING CONCRETE DURING FREEZING OR NEAR-FREEZING WEATHER. THE RECOMMENDED PROCEDURES LISTED IN ACI 306: COLD WEATHER CONCRETING SHALL BE FOLLOWED.</p> <ul style="list-style-type: none"> i COLD WEATHER IS DEFINED AS A PERIOD WHEN, FOR MORE THAN 3 CONSECUTIVE DAYS, THE FOLLOWING CONDITIONS EXIST: <ul style="list-style-type: none"> (a) THE AVERAGE DAILY AIR TEMPERATURE IS LESS THAN 40-DEGREES F AND (b) THE AIR TEMPERATURE IS NOT GREATER THAN 50-DEGREES F FOR MORE THAN ONE-HALF OF ANY 24-HOUR PERIOD. <p>J. HOT WEATHER REQUIREMENTS. DURING HOT WEATHER, PROPER ATTENTION SHALL BE GIVEN TO INGREDIENTS, PRODUCTION METHODS, HANDLING, PLACING, PROTECTION, AND CURING TO PREVENT EXCESSIVE CONCRETE TEMPERATURES OR WATER EVAPORATION THAT COULD IMPAIR REQUIRED STRENGTH OR SERVICEABILITY OF THE MEMBER OR STRUCTURE. THE RECOMMENDED PROCEDURES LISTED IN ACI 305: HOT WEATHER CONCRETING SHALL BE FOLLOWED.</p> <ul style="list-style-type: none"> i HOT WEATHER IS ANY COMBINATION OF THE FOLLOWING CONDITIONS THAT TENDS TO IMPAIR THE QUALITY OF FRESHLY MIXED OR HARDENED CONCRETE BY ACCELERATING THE RATE OF MOISTURE LOSS AND RATE OF CEMENT HYDRATION, OR OTHERWISE CAUSING DETERIMENTAL RESULTS: <ul style="list-style-type: none"> (a) HIGH AMBIENT TEMPERATURE. (b) HIGH CONCRETE TEMPERATURE. (c) LOW RELATIVE HUMIDITY. (d) WIND SPEED. (e) SOLAR RADIATION. 	<p>A. PLACEMENT OF ALL REINFORCING STEEL WITHIN CONCRETE STRUCTURES SHALL BE IN CONFORMANCE WITH ACI 318/350.</p> <p>B. REINFORCING STEEL HOOKS, BENDS, TIES, SPLICES AND OTHER REINFORCEMENT DETAILS SHALL BE IN ACCORDANCE WITH ACI 315, GUIDE TO PRESENTING REINFORCING STEEL DESIGN DETAILS.</p> <p>C. SPACING LIMITS FOR REINFORCEMENT SHALL BE IN CONFORMANCE WITH ACI 318/350.</p> <p>D. CONCRETE PROTECTION FOR REINFORCEMENT. UNLESS NOTED ELSEWHERE ON THE DRAWINGS, ALL REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER:</p> <ul style="list-style-type: none"> i FOR NON-LIQUID CONTAINING CONCRETE STRUCTURES; PER ACI 318: <ul style="list-style-type: none"> (a) CONCRETE CAST AGAINST EARTH: 3.00 INCH (b) CONCRETE EXPOSED TO EARTH OR WEATHER: <ul style="list-style-type: none"> • NO. 5 OR SMALLER BARS: 1.50-INCH • NO. 6 OR LARGER BARS: 2.00-INCH (c) CONCRETE NOT EXPOSED TO EARTH OR WEATHER: <ul style="list-style-type: none"> • NO. 11 OR SMALLER BARS: 0.75-INCH • NO. 14 OR LARGER BARS: 1.50-INCH (d) BEAMS AND COLUMNS: <ul style="list-style-type: none"> • PRIMARY REINFORCEMENT, TIES, STIRRUPS, OR SPIRALS: 1.50-INCH <p>ii FOR LIQUID CONTAINING CONCRETE STRUCTURES; PER ACI 350:</p> <ul style="list-style-type: none"> (a) CONCRETE CAST AGAINST EARTH: 3.00 INCH (b) CONCRETE EXPOSED TO EARTH, LIQUID OR WEATHER: <ul style="list-style-type: none"> • SLABS AND JOINTS: 2.00-INCH • WALLS: 2.00-INCH (c) BEAMS AND COLUMNS: <ul style="list-style-type: none"> • TIES, STIRRUPS, OR SPIRALS: 2.00-INCH • PRIMARY REINFORCEMENT: 2.50-INCH (d) FOOTINGS AND BASE SLABS: <ul style="list-style-type: none"> • FORMED SURFACES: 2.00-INCH • TOP OF FOOTINGS AND BASE SLABS: 2.00-INCH <p>E. CONCRETE BLOCKS OR PLASTIC-COATED BAR CHAIRS SHALL BE PROVIDED FOR SUPPORT OF ALL SLAB REINFORCING STEEL, SUFFICIENT IN NUMBER TO PREVENT SETTLEMENT OR SAGGING, BUT IN NO CASE SHALL SUCH SUPPORT BE CONTINUOUS. METAL CLIPS OR SUPPORTS SHALL NOT BE PLACED IN CONTACT WITH THE FORMS OR THE SUB-GRADE.</p> <p>F. DOWELS AND ANCHOR BOLTS SHALL BE WIRED OR OTHERWISE HELD IN CORRECT POSITION PRIOR TO PLACING CONCRETE. CARE SHALL BE TAKEN TO ENSURE THAT DOWELS AND ANCHOR BOLTS REMAIN PLUM AFTER CONCRETE IS POURED AND VIBRATED. IN NO CASE SHALL DOWELS OR ANCHOR BOLTS BE STABBED INTO FRESHLY POURED CONCRETE!</p> <p>G. PROVIDE DOWELS IN FOOTINGS AND AT CONSTRUCTION JOINTS TO MATCH VERTICAL REINFORCING BAR SIZE AND SPACING, UNLESS OTHERWISE NOTED ON THE DRAWINGS.</p> <p>H. COORDINATE PLACEMENT OF DOWELS INTO MASONRY OR BRICK WALLS WITH THE MASONRY SHOP DRAWINGS.</p> <p>I. WHERE DRILLED IN ANCHORS ARE TO BE POST-INSTALLED INTO CONCRETE SURFACES TAKE CARE TO LOCATE REINFORCING STEEL SO THAT IT WILL NOT INTERFERE WITH THE DRILLING OPERATIONS. MOVE BARS PLUS OR MINUS 1 TO 2 INCHES IN ORDER TO AVOID DRILLING CONFLICTS.</p> <p>J. ALL BAR BENDS, HOOKS, SPLICES AND OTHER REINFORCING STEEL DETAILS SHALL CONFORM TO THE REQUIREMENTS OF ACI 315.</p> <p>K. UNLESS OTHERWISE NOTED ON THE PLANS ALL BARS SHALL BE SPLICED WITH A MINIMUM CLASS B LAP SPLICE; LAP SPLICES OF DEFORMED BARS AND DEFORMED WIRE IN COMPRESSION ZONES MAY BE CLASS A SPLICES.</p> <p>L. AT ALL CORNERS AND WALL INTERSECTIONS PROVIDE BENT BARS TO MATCH THE HORIZONTAL REINFORCING STEEL AND IN ACCORDANCE WITH THE TYPICAL CORNER REINFORCING DETAILS.</p> <p>M. CHAMFER ALL EXPOSED CORNERS AND FILLET ENTRANT ANGLES 3/4" UNLESS OTHERWISE NOTED ON THE DRAWINGS.</p> <p>N. WATERSTOP. ALL CONTROL AND CONSTRUCTION JOINTS IN LIQUID-RETAINING STRUCTURES SHALL BE DOWELED, KEYED AND PROVIDED WITH CONTINUOUS WATERSTOP, PER THE TYPICAL DETAILS, TECHNICAL SPECIFICATIONS OR AS DIRECTED BY THE PROJECT ENGINEER.</p> <p>O. AT SLAB AND WALL OPENINGS PROVIDE A MINIMUM OF (4) #5 BARS: OVER, UNDER AND AT EITHER SIDE OF THE OPENINGS. EXTEND THESE BARS A MINIMUM OF 24" PAST THE OPENING EDGE. PROVIDE (1) MATT OF (4) #5 BARS FOR WALLS OR SLABS WITH SINGLE-LAYER REINFORCING AND (2) MATTS OF (4) #5 BARS FOR DOUBLE-LAYER REINFORCING WALLS OR SLABS. PROVIDE #4, 4'-0" LONG DIAGONAL BARS AT EACH RE-ENTRANT CORNER IN SLABS; (1) BAR FOR SLABS WITH SINGLE LAYER REINFORCING AND (2) BARS FOR SLABS WITH DOUBLE LAYER REINFORCING.</p>	<p>REUSE OF DRAWINGS J-U-B SHALL RETAIN ALL COMMON LAW, STATUTORY, COPYRIGHT AND OTHER RESERVED RIGHTS OF THESE DRAWINGS AND THE SAME SHALL NOT BE REUSED WITHOUT J-U-B'S PRIOR WRITTEN CONSENT. SOLICITATION AND WITHOUT LIABILITY OR LEGAL EXPENSE TO J-U-B. REVISION</p> <p>NO. DESCRIPTION BY API DATE</p>					
<p>JACKSON CANYON VIEW SD ANALYSIS OGDEN CITY</p>		<p>GENERAL STRUCTURAL NOTES</p>					

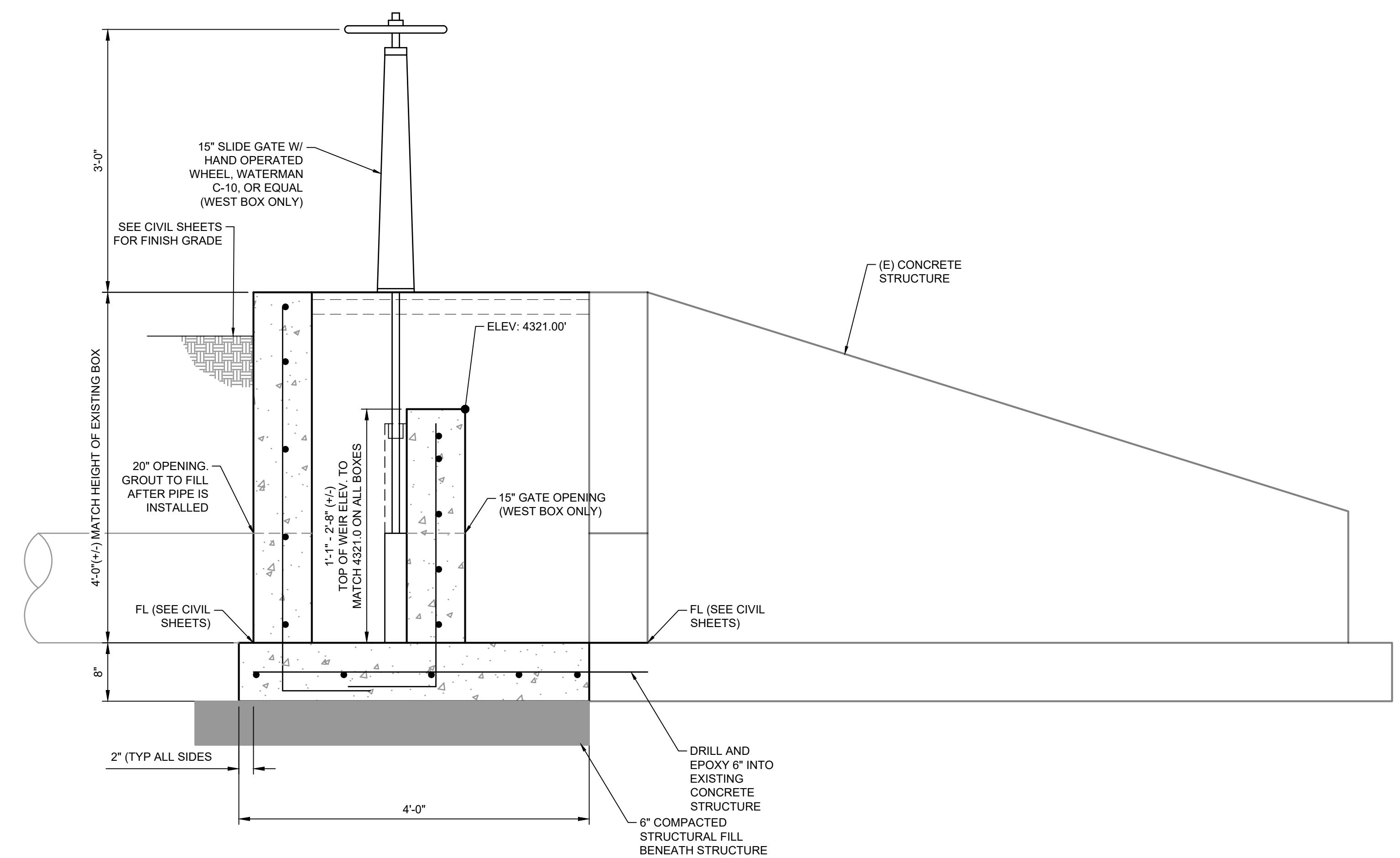
B1 OUTFALL STRUCTURE PLAN
SCALE: 1" = 1'-0"

B1 SCALE 1" = 1'-0"



D1 OUTFALL STRUCTURE SECTION
SCALE: 1" = 1' 0"

D1 COFFEE



REUSE OF DRAWINGS	REVISION						
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JACKSON CANYON VIEW SD ANALYSIS
OGDEN CITY

PLAN AND SECTION

FILE : 55-25-020 S-101X
JUB PROJ. #: 55-25-020
DRAWN BY: EM
DESIGN BY: JCH
CHECKED BY: ----

ONE INCH
AT FULL SIZE, IF NOT ONE
INCH, SCALE ACCORDINGLY

LAST UPDATED: 1/29/2026

SHEET NUMBER:
S-101

